

#### REPORT

# Cascadia Phase 4 - East Viterra Siding Expansion

Geotechnical Site Investigation Report

Submitted to:

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# **1.0 INTRODUCTION**

Golder Associates Ltd. (Golder) has been retained by Canadian Pacific Railway (CP) to provide site investigation and consulting geotechnical engineering services to support Hatch with the design of the proposed track expansion for the Cascadia Phase 4 Viterra East Siding Expansion Program located in Burnaby, British Columbia (the Site). The purpose of the geotechnical drilling program was to investigate the subsurface conditions at the site to support the route option selection and preliminary design.

## 1.1 Terms of Reference

During the drilling investigation, the Phase 4 Viterra East Siding Expansion Program was in the route option selection and optimization phase, with 30% Design Progress and Class 3 Cost Estimate targeted for mid-October. The scope of work was developed following consultation with Hatch and CP, with the final scope set out in the Golder Desktop Study and Work Plan dated 27 August 2020 (Golder, 2020). The scope of work was conducted in accordance with the terms and conditions in the Supply of Services Agreement, Agreement No. 5600020102 executed 31 January 2018 and Purchase Order 4200021331 dated 1 June 2020.

# 1.2 Scope of Report

The investigation scope consisted of two drilling programs; an onshore drilling investigation within the footprint of the existing northern track and an intertidal drilling investigation on the shoreline along the eastern segment of the proposed track expansion. A key plan of the site area is shown in Figure 1.

It should be noted that the test hole locations were estimated using handheld GPS, and they were not accurately surveyed; as such, the locations and elevations are approximate. The methods used to determine the elevations are described in the report. Approximate test hole locations are presented in Figure 2 and tabulated in APPENDIX A.

The scope of this report is limited to the documentation of the factual results of the geotechnical investigation and does not include the testing or assessment of potential soil and/or groundwater contamination, archaeological, geophysical, biological or bio-environmental services for the project. This report contains only factual results of the geotechnical investigation program and, specifically, does not include site characterization, geotechnical or geological interpretation of factual data, nor engineering assessment/recommendations, which will be presented in a subsequent report.

This report should be read in conjunction with the "**Information and Limitations of This Report**" which is included following the text of this report. The reader's attention is specifically drawn to this information, as it is essential that it is followed for the proper use and interpretation of this report.

# 2.0 PROPOSED ROUTE ALIGNMENTS

The original conceptual extension alignment options were presented to Golder through email correspondence and a project team meeting on 11 May 2020. Updated conceptual options and schematics were provided to Golder on 16 August 2020 (Hatch, 2020 (a)).

In general, the alignment options as described by Hatch are as follows:

- Option 1: North Alignment Original Concept for proposed extension being completely to the north of the existing tracks
- Option 2: South-North Alignment alignment extension starts south at Mile 123.3, and shifts to the north to the west of the Parkland Refinery near Mile 123.8
- Option 3: North-South-North Alignment alignment extension starts north at Mile 123.3, then shifts south east of the Parkland Refinery near Mile 123.5, and switches back to North after passing the Parkland Refinery near Mile 123.8

This site investigation was prepared considering Option 1 (Hatch, 2020 (c)) as the preferred alignment on the 11 May 2020 drawings.

The proposed investigation for the Option 1 alignment was sent to CP for approval on 6 October 2020 and a plan drawing of the proposed alignment option was provided to Golder on 15 October 2020 (Hatch, 2020 (b)).

# 3.0 GEOTECHNICAL SITE INVESTIGATION

### 3.1 Field Mapping

Field mapping of the project area was carried out by two geotechnical engineering personnel from Golder on 2 June 2020. Weather was cloudy and cool with rain developing in the afternoon. Staff traversed the full length of the Site along the rail and foreshore beach below the rail line. The purpose of the mapping was to observe exposures of surficial materials and refine our understanding of the conditions at the site scale. The results of the field mapping are presented in Golder's Desktop Study and Work Plan technical memorandum dated 27 August 2020 (Golder, 2020).

## 3.2 Site Investigation Program

A total of eleven drilling locations (BH20-01 to -09, SH20-10 and -11) were conducted as part of the site investigation. The site investigation program consisted of nine onshore air-rotary test holes drilled between 23 June 2020 and 9 July 2020 and two intertidal test holes drilled on 27 July 2020 and 28 July 2020. Selected samples from the test holes and were subjected to a variety of laboratory testing to assist with classification and characterization of the subsurface strata. A summary of the test hole details is tabulated in APPENDIX A.

The site investigation included the following:

- Site reconnaissance and field mapping
- Non-intrusive utility locating in the vicinity of proposed test hole locations
- Drilling and sampling of soil within test holes, utilizing sonic and air-rotary drilling techniques
- Split-spoon sampling and penetration test observations
- Torque wrench shear vanes

### 3.3 Permits and Access Agreements

#### 3.3.1 CP Access and Permissions

CP permissions, track protection and hi-rail access were coordinated for BH20-01 to BH20-09 through the CP project team and the operations and maintenance department. By necessity, the onshore test holes were drilled within the northern track between rail ties due to the requirement to maintain rail traffic on the southern track. Both "Rule 42" and Track Occupancy Permits (TOP) were utilized situationally along the northern track. After drilling activities were conducted, all equipment was taken offsite and the drilling locations were left in a similar condition as observed during the pre-work reconnaissance.

#### 3.3.2 Vancouver Fraser Port Authority

A Vancouver Fraser Port Authority (VFPA) Project Permit was applied for and obtained by Hemmera Envirochem Inc. (Hemmera) on behalf of CP (Project Permit 20-067). A licence was obtained from the VFPA on 24 July 2020 as part of VFPA permit requirements. Golder submitted a marine construction and staging plan to the VFPA for review and received approval on 20 July 2020. Notifications were also sent to the Harbour Master, VFPA Environmental Programs and CCG Marine Communications and Traffic Services on 21 July 2020.

# 3.4 Health, Safety and Environment Plan

Golder created an investigation Health, Safety and Environment Plan (HASEP) procedures and template, which was updated with project specific contacts and information, and submitted for review by Hatch.

The HaSEP included the following:

- Roles and responsibilities of all personnel and sub-contractors including identification of Site Supervisor responsible for coordination and management of Health and Safety on the site and appropriate First Aid personnel and responsibilities
- Communication protocols
- B Hazards and sensitive features related to investigative work and control measures to manage risk
- Minimum levels of personal protective equipment and other protection requirements
- Environmental Emergency Response Procedure
- Job Safety and Environmental Analysis (JSEA), and daily tailgate meetings

The HaSEP Revision for intertidal program included the following:

- Environmental Protection Plan (MudBay Drilling Ltd.)
- Standard operating procedures (MudBay Drilling Ltd.)
- Working on and around water

# 3.5 Utility Clearance

Prior to commencement of the subsurface investigations, underground utility and service information obtained from BC One Call, DigShaw and Metro Vancouver were reviewed. GeoScan Subsurface Surveys Inc. of Burnaby, BC was retained by Golder to locate underground utilities near the proposed hole locations. Locates were carried out using non-intrusive methods that included Electro-Magnetic (EM) locating and ground penetrating radar (GPR). For the intertidal investigation, larger areas were scanned for utilities to allow more flexibility to move the test hole locations based on access issues. Identified buried service locations were marked on the ground with spray paint, selected final hole locations were cleared, and approximate electronic drawings were provided to Golder.

# 3.6 Drilling Inspection and Test Hole Location Identification

The onshore air-rotary drilling program was conducted by Geotech Drilling Ltd. (Geotech) and the intertidal sonic drilling program was conducted by MudBay Drilling Ltd. (MudBay). Both programs were carried out under the full-time inspection of a member of Golder's geotechnical staff, who identified the test hole locations in the field, logged the subsurface conditions encountered, and collected soil samples and soil core for detailed examination and laboratory testing. A record of the soil conditions encountered, as well as test hole closure details, can be found in the Record of Sonic Hole and Record of Borehole sheets in APPENDIX B.

All test hole locations are approximate only and based on handheld GPS co-ordinates taken in the field combined with measured distances to identifiable site features such as rail tracks. Locations were planned and identified in the field before the investigation by both Golder and Hatch. No detailed survey locations or elevation measurements were established in the field following the drilling investigation. In this report, all depths are referenced to ground surface at the time of drilling.

Field identification and classification of soils was carried out in accordance with Golder's soil classification system (Golder, 2018) which is generally consistent with the concepts presented in ASTM International (ASTM) D2487 and D2488 and the Canadian Foundation Engineering Manual (CFEM, 2017), with some differences intended to improve the compatibility of the soil descriptions with the material geotechnical engineering behaviour. This system is generally consistent with the Unified Soil Classification System (USCS). Coarse-grained soils are classified based on USCS, and are described in terms of relative proportions of the mineral constituents; whereas, fine grained soils are classified with reference to Golder's soil classification system, and are based on plasticity (Atterberg limits testing) with reference to the ASTM D2487 plasticity chart. A summary of the key aspects of Golder's soil classification system is presented on the "Geotechnical Soil Description Terminology" fly sheets, which immediately precede the Record of Sonic Hole and Record of Borehole sheets in APPENDIX B.

## 3.7 Drilling Program

#### 3.7.1 Data Presentation

The data collected during the 2020 drilling and laboratory testing program are summarized in the appendices:

- APPENDIX A: Summary Tables
- APPENDIX B: Record of Borehole and Record of Sonic Hole sheets
- APPENDIX C: Laboratory Testing Results
- APPENDIX D: Energy Measurements on Standard Penetration Test Hammer

#### 3.7.2 ODEX Air-Rotary Drilling and Sampling

The nine ODEX air-rotary drilled test holes were put down using a Fraste track-mounted drill rig owned and operated by Geotech Drilling Services Ltd. of Delta, BC.

ODEX air-rotary drilling employed a two-piece system comprising casing and a downhole hammer driven by central rods. The hammer is inserted through the casing and the cutting head is exposed at the bottom of the casing. A small cam deploys a crescent-shaped bit that cuts ahead of the casing in order to reduce friction on the casing. Compressed air is cycled through the hammer, driving the percussion aspect of the hammer which breaks apart the overburden encountered during advancement. As the compressed air is cycled through the hammer out the tip, the cuttings at the bottom of the hole are brought up to ground surface by forced air. During advancement of the hammer, the casing is also driven with the head of the hammer. At selected intervals, the hammer is withdrawn from the casing to allow for intermittent sampling through the casing, such as using split-spoon samplers.

## 3.7.3 Sonic Drilling and Sampling

The two sonic cored test holes were conducted using a LS250 mini rubber tracked drilling rig with sonic drilling capabilities. The drilling rig was owned and operated by Mud Bay and drilling generally followed ASTM D6914 procedures.

Sonic drilling utilized both an open-faced bit and dual-cased single-tube core barrel system that employs high frequency mechanical vibration to advance the drill string and obtain continuous or near-continuous disturbed core samples of the soils, and in some cases, bedrock. The drilling technique involves vibrating the entire drill string at a frequency ranging between 50 and 150 cycles per second, adjusted during operation to suit the ground conditions encountered. The technique employs low-speed rotational motion, coupled with downward pressure, to advance the drill string. Sonic cored test hole advancement is achieved through the process of fracturing, shearing, and displacement depending on the type and consistency of the material encountered; given this process, the retrieved core is disturbed, sometimes with substantial changes to the original volume, and furthermore depths calculated from lengths of soil core may not be representative of logged feature locations.

During typical Sonic core drilling operations, the soil enters the 102 mm diameter inner core barrel. Upon completion of each drill run, the outer steel casing is advanced to the end of the run, the core barrel and drill rods are removed, and the sonic core sample is vibrated out of the core barrel directly into a plastic sample bag which is larger than the core diameter, before being transferred into wooden core boxes. Sonic vibration from the drilling method does cause some disturbance of the soil structure and can destroy secondary structure features of the strata in the core samples that are retrieved. In addition, heating of the core barrel during vibration and rotation may modify the in-situ moisture content of the core sample obtained. Both the core barrel and casing shoes have water jets to allow for cooling and lubrication of the bit, and to flush cuttings out of the hole. Where practical, advancement of the drill string during this investigation program was typically carried out without addition of fluid to prevent additional disturbance and washing of the soils. Advancement of the casing behind the core barrel was typically carried out with the assistance of drill fluids.

Soils can also be displaced if the core sampler barrel becomes blocked (e.g., by cobbles) during the advancement, causing displacement of the soils and poor recovery. In addition, as a result of the high frequency vibration, the sonic drilling method tends to pulverize (i.e., fragment) or core through boulders and large cobble-sized particles while advancing through the soil strata.

#### 3.7.4 Standard Penetration Testing

Soil sampling was carried out at selected intervals using intermittent split-spoon sampling techniques with a 50 mm or 76 mm diameter split-soon sampling tube and a 63.6 kg, 760 mm drop safety hammer. The split-spoon sampler was threaded onto AWJ drill rods (35 mm OD diameter) or NWJ drill rods (67 mm OD diameter). Upon retrieval of the sampler, the split-tube was opened, and the recovered soil was classified, photographed, stored in labeled plastic bags, and transported to the Golder Burnaby laboratory facility for detailed examination and index testing of selected samples.

The split-spoon sampling was carried out in general accordance with ASTM D1586, Standard Test Method for Standard Penetration Test (SPT) and Split-Barrel Sampling of Soils; however, it should be noted that conducting penetration testing within a test hole advanced with ODEX air-rotary or sonic methods does not strictly comply with ASTM standards due to the vibration and potential disturbance resulting from these methods.

The recorded blow counts for the individual penetration tests are presented on the Record of Borehole and Record of Sonic Hole sheets. The relative density or consistency reported in the logs were generally based on the measured blow counts. It should be noted that in certain strata (i.e., where gravel or cobble-sized particles were encountered), the recorded blow counts may not be representative of the relative density or stiffness of the soil matrix, and a combination of engineering judgement and laboratory testing is required to evaluate the compactness/consistency of the material. The reader is also cautioned to consider overburden and energy loss effects (as well as other correction factors) when interpreting raw penetration test results.

Caution and judgement should therefore be exercised in the interpretation of the recorded blow counts presented on the Record of Borehole and Record of Sonic Hole sheets and their correlation with standard or corrected "N" values.

#### 3.7.4.1 Torque Wrench Shear Vanes

Torque wrench shear vane tests were conducted at selected intervals within fine-grained soils within test holes SH20-10 and 11. The Precision Instruments vane testing apparatus model No. D3F250FMP, serial No. 1011 was owned and operated by MudBay, and calibration record No. 510127 was completed by McCANN Equipment Ltd. The field vane consists of a type 2, 5 cm x 10 cm vane head. A summary of the depths and testing results are presented in APPENDIX A.

The shear vane tests were carried out in general accordance with ASTM D2573/D2573M; however, it should be noted that conducting shear vane testing within a test hole advanced with ODEX air-rotary or sonic methods does not strictly comply with ASTM standards, due to the vibration and disturbance caused from air rotary and sonic drilling caution and judgement should be exercised in the interpretation of shear strength data presented on the test hole records.

#### 3.7.5 Energy Measurements on Penetration Test Hammer

Energy measurement documentation was obtained from Mud Bay for the hammer equipment that was utilized onsite during the intertidal drilling. The energy measurements were used to correlate a work efficiency of the hammer used during the drilling investigation. Using the hammer efficiency information, the observed blow counts can be converted into equivalent SPT N blow counts for use in engineering analyses and design.

No energy measurement documentation was obtained from Geotech Drilling for the hammer equipment that was utilized during the onshore investigation.

All energy measurement information was provided by drilling subcontractors, and not conducted by Golder during the investigation. Available energy measurements of the SPT hammer efficiencies are provided in APPENDIX C of this report.

#### 3.7.6 Test Hole Closure

The test holes were sealed using bentonite chips in accordance with BC Water Sustainability Act requirements. The ground surface was restored with ballast for the onshore test holes and sand for the intertidal test holes. Hole completion details for all test holes are documented in APPENDIX B.

# 4.0 LABORATORY TESTING SUMMARY

Laboratory soil index testing was undertaken on selected soil samples obtained from grab samples and the splitspoon samples retrieved on site. Soil index testing was conducted at Golder's Burnaby laboratory and in accordance with ASTM standards as follows:

- ASTM D2216 Standard Test Method for Determination of Water (Moisture) Content of Soil and Rock by Mass
- ASTM D6913 Standard Test Method for Particle-Size Distribution (Gradation) of Soils Using Sieve Analysis
- ASTM D7928 Standard Test Method for Particle-Size Distribution (Gradation) of Fine-Grained Soils Using the Sedimentation (Hydrometer) Analysis
- ASTM D4318 Standard Test Methods for Liquid Limit, Plastic Limit, and Plasticity Index of Soils

Sample preparation was carried out in general conformance with ASTM D421. It is important to recognize that the sampling methods in the field limit the particle sizes that can be recovered from the test holes. As such, the laboratory gradation test results shown reflect samples that are entirely smaller than these maximum sizes, and may not be representative of boulder, cobble, or coarse gravel content. Laboratory soil index test results are included in APPENDIX D.

# 5.0 CLOSURE

We trust that the geotechnical data provided herein meets your present requirements. Should you have any questions regarding the above, please do not hesitate to contact us.

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# **IMPORTANT INFORMATION AND LIMITATIONS OF THIS REPORT**

**Standard of Care:** Golder Associates Ltd. (Golder) has prepared this report in a manner consistent with that level of care and skill ordinarily exercised by members of the engineering and science professions currently practising under similar conditions in the jurisdiction in which the services are provided, subject to the time limits and physical constraints applicable to this report. No other warranty, expressed or implied is made.

**Basis and Use of the Report:** This report has been prepared for the specific site, design objective, development and purpose described to Golder by the Client. The factual data, interpretations and recommendations pertain to a specific project as described in this report and are not applicable to any other project or site location. Any change of site conditions, purpose, development plans or if the project is not initiated within eighteen months of the date of the report may alter the validity of the report. Golder can not be responsible for use of this report, or portions thereof, unless Golder is requested to review and, if necessary, revise the report.

The information, recommendations and opinions expressed in this report are for the sole benefit of the Client. No other party may use or rely on this report or any portion thereof without Golder's express written consent. If the report was prepared to be included for a specific permit application process, then upon the reasonable request of the client, Golder may authorize in writing the use of this report by the regulatory agency as an Approved User for the specific and identified purpose of the applicable permit review process. Any other use of this report by others is prohibited and is without responsibility to Golder. The report, all plans, data, drawings and other documents as well as all electronic media prepared by Golder are considered its professional work product and shall remain the copyright property of Golder, who authorizes only the Client and Approved Users to make copies of the report, but only in such quantities as are reasonably necessary for the use of the report by those parties. The Client and Approved Users may not give, lend, sell, or otherwise make available the report or any portion thereof to any other party without the express written permission of Golder. The Client acknowledges that electronic media is susceptible to unauthorized modification, deterioration, and incompatibility and therefore the Client cannot rely upon the electronic media versions of Golder's report or other work products.

The report is of a summary nature and is not intended to stand alone without reference to the instructions given to Golder by the Client, communications between Golder and the Client, and to any other reports prepared by Golder for the Client relative to the specific site described in the report. In order to properly understand the suggestions, recommendations and opinions expressed in this report, reference must be made to the whole of the report. Golder can not be responsible for use of portions of the report without reference to the entire report.

Unless otherwise stated, the suggestions, recommendations and opinions given in this report are intended only for the guidance of the Client in the design of the specific project. The extent and detail of investigations, including the number of test holes, necessary to determine all of the relevant conditions which may affect construction costs would normally be greater than has been carried out for design purposes. Contractors bidding on, or undertaking the work, should rely on their own investigations, as well as their own interpretations of the factual data presented in the report, as to how subsurface conditions may affect their work, including but not limited to proposed construction techniques, schedule, safety and equipment capabilities.

**Soil, Rock and Groundwater Conditions:** Classification and identification of soils, rocks, and geologic units have been based on commonly accepted methods employed in the practice of geotechnical engineering and related disciplines. Classification and identification of the type and condition of these materials or units involves judgment, and boundaries between different soil, rock or geologic types or units may be transitional rather than abrupt. Accordingly, Golder does not warrant or guarantee the exactness of the descriptions.

Special risks occur whenever engineering or related disciplines are applied to identify subsurface conditions and even a comprehensive investigation, sampling and testing program may fail to detect all or certain subsurface conditions. The environmental, geologic, geotechnical, geochemical and hydrogeologic conditions that Golder interprets to exist between and beyond sampling points may differ from those that actually exist. In addition to soil variability, fill of variable physical and chemical composition can be present over portions of the site or on adjacent properties. **The professional services retained for this project include only the geotechnical aspects of the subsurface conditions at the site, unless otherwise specifically stated and identified in the report.** The presence or implication(s) of possible surface and/or subsurface contamination resulting from previous activities or uses of the site and/or resulting from the introduction onto the site of materials from off-site sources are outside the terms of reference for this project and have not been investigated or addressed.

Soil and groundwater conditions shown in the factual data and described in the report are the observed conditions at the time of their determination or measurement. Unless otherwise noted, those conditions form the basis of the recommendations in the report. Groundwater conditions may vary between and beyond reported locations and can be affected by annual, seasonal, and meteorological conditions. The condition of the soil, rock and groundwater may be significantly altered by construction activities (traffic, excavation, groundwater level lowering, pile driving, blasting, etc.) on the site or on adjacent sites. Excavation may expose the soils to changes due to wetting, drying or frost. Unless otherwise indicated the soil must be protected from these changes during construction.

**Sample Disposal:** Golder will dispose of all uncontaminated soil and/or rock samples 90 days following issue of this report or, upon written request of the Client, will store uncontaminated samples and materials at the Client's expense. In the event that actual contaminated soils, fills, or groundwater are encountered or are inferred to be present, all contaminated samples shall remain the property and responsibility of the Client for proper disposal.

**Follow-Up and Construction Services:** All details of the design were not known at the time of submission of Golder's report. Golder should be retained to review the final design, project plans and documents prior to construction, to confirm that they are consistent with the intent of Golder's report.

During construction, Golder should be retained to perform sufficient and timely observations of encountered conditions to confirm and document that the subsurface conditions do not materially differ from those interpreted conditions considered in the preparation of Golder's report and to confirm and document that construction activities do not adversely affect the suggestions, recommendations and opinions contained in Golder's report. Adequate field review, observation and testing during construction are necessary for Golder to be able to provide letters of assurance, in accordance with the requirements of many regulatory authorities. In cases where this recommendation is not followed, Golder's responsibility is limited to interpreting accurately the information encountered at the borehole locations, at the time of their initial determination or measurement during the preparation of the Report.

**Changed Conditions and Drainage:** Where conditions encountered at the site differ significantly from those anticipated in this report, either due to natural variability of subsurface conditions or construction activities, it is a condition of this report that Golder be notified of any changes and be provided with an opportunity to review or revise the recommendations within this report. Recognition of changed soil and rock conditions requires experience and it is recommended that Golder be employed to visit the site with sufficient frequency to detect if conditions have changed significantly.

Drainage of subsurface water is commonly required either for temporary or permanent installations for the project. Improper design or construction of drainage or dewatering can have serious consequences. Golder takes no responsibility for the effects of drainage unless specifically involved in the detailed design and construction monitoring of the system.



PROJECT NO.

20143647

CONTROL

3000

COORDINATE SYSTEM: NAD 1983 UTM ZONE 10N

ABLENERGE(G) 1. BASEMAP OBTAINED FROM ESRI. USED UNDER LICENCE, ALL RIGHTS RESERVED. 2. NATIONAL ROAD NETWORK CONTAINS INFORMATION LICENSED UNDER THE OPEN GOVERNMENT LICENCE – CANADA. 3. MUNICIPAL BOUNDARY LICENSED UNDER OPEN GOVERNMENT LICENSE - BRITISH COLUMBIA.

REVIEWED J. SOQUILA APPROVED J. RICHMOND REV. 0

FIGURE

1



NNED BOREHOLES	

CONSULTANT	YYYY-MM-DD	2020-11-19
	DESIGNED	B. HOLZMAN
	PREPARED	L. HOLMES
	REVIEWED	J. SOQUILA

APPENDIX A

# **Summary Tables**

#### APPENDIX A Table 1: Summary of Geotechnical Test Holes Phase 4 Vieterra East Siding Van Group

	Plan Test Hole ID Drilling Date(s) Test		Planning Drilling		GPS Coordinates <sup>1,2</sup>		Approximate	
Test Hole ID			Method	Northing (m)	Easting (m)	Elevation (m) <sup>3</sup>	mileage	Drilled depth (m)
BH20-01	June 25, 2020	В	ODEX	5459886	499380	5.8	123.89	4.98
BH20-02	June 27, 2020	D	ODEX	5459772	499538	5.8	123.75	6.55
BH20-03	June 27, 2020	F	ODEX	5459725	499767	5.8	123.62	5.13
BH20-04	June 28, 2020 to July 7, 2020	G	ODEX	5459741	499982	6.0	123.47	12.80
BH20-05	July 3, 2020	н	ODEX	5459831	500131	6.2	123.35	10.67
BH20-06	July 5, 2020	E	ODEX	5459739	499605	5.6	123.71	9.37
BH20-07	July 5, 2020	А	ODEX	5459974	499190	6.0	124.01	5.03
BH20-08	July 8, 2020	С	ODEX	5459810	499486	5.9	123.8	7.62
BH20-09	July 9, 2020	М	ODEX	5460031	500440	6.2	123.14	11.28
SH20-10	July 27, 2020	I	Sonic	5459739	499932	2.0	123.51	21.34
SH20-11	July 28, 2020	L	Sonic	5459844	500134	1.0	123.35	15.24

<sup>1</sup> All coordinates provided are referenced to UTM NAD83, Zone 10U.

<sup>2</sup> Coordinates were obtained using a hand-held GPS typically accurate to within 5 m.

<sup>3</sup> Elevations were estimated from surface profiles provided by Hatch.

### APPENDIX A Table 2: Summary of Torque Wrench Shear Vanes Phase 4 Vieterra East Siding Van Group

	onic Hole ID Sample No. Approximate Tip Depth (mbgs)			Measure	d Torque	Vane Shear S	Strength (kPa)
Sonic Hole ID			Vane Type	Peak (ft*lbs)	Remolded (ft*lbs)	Peak (kPa)	Remolded (kPa)
SH20-10	Vane 1	3.51	Type 2 - 5 cm x 10 cm	100	50	267.1	133.6
SH20-10	Vane 2	6.25	Type 2 - 5 cm x 10 cm	160	25	427.4	66.8
SH20-10	Vane 3	9.30	Type 2 - 5 cm x 10 cm	75	30	200.3	80.1
SH20-10	Vane 4	15.39	Type 2 - 5 cm x 10 cm	125	25	333.9	66.8
SH20-10	Vane 5	18.44	Type 2 - 5 cm x 10 cm	180	75	480.8	200.3
SH20-11	Vane 1	3.20	Type 2 - 5 cm x 10 cm	130	30	347.2	80.1
SH20-11	Vane 2	4.72	Type 2 - 5 cm x 10 cm	145	25	387.3	66.8
SH20-11	Vane 3	6.25	Type 2 - 5 cm x 10 cm	135	15	360.6	40.1
SH20-11	Vane 4	9.30	Type 2 - 5 cm x 10 cm	120	30	320.5	80.1

APPENDIX B

Record of Borehole and Record of Sonic Hole Sheets

Organic or Inorganic	Soil Group	Туре	of Soil	Gradation or Plasticity	Cu	$=\frac{D_{60}}{D_{10}}$		$Cc = \frac{(D_{30})^2}{D_{10}xD_{60}}$		Organic Content	USCS Group Symbol	Group Name
	<u> </u>	Gravels To <u>s</u> G G G G G G G C G C G C S C S C S C S C		Poorly Graded		<4		≤1 or ≥	:3		GP	GRAVEL
(ss)	5 mm	VELS / mas raction	fines (by mass)	Well Graded		≥4		1 to 3	3		GW	GRAVEL
, by ma	SOILS an 0.07	GRA 50% by oarse fr	Gravels with	Below A Line			n/a				GM	SILTY GRAVEL
GANIC it ≤30%	AINED arger th	(> cc larc	fines (by mass)	Above A Line			n/a			≤30%	GC	CLAYEY GRAVEL
INOR	SE-GR ss is la	of is	Sands with	Poorly Graded		<6		≤1 or ≩	≥3		SP	SAND
rganic (	COARS by ma	VDS / mass raction n 4.75	fines (by mass)	Well Graded		≥6		1 to 3	3		SW	SAND
0)	(>50%	SAI 50% by oarse f	Sands with	Below A Line			n/a				SM	SILTY SAND
		(≥ sma	fines (by mass)	Above A Line			n/a				SC	CLAYEY SAND
Organic	Soil	Turno	of Soil	Laboratory		F	ield Indic	ators	Toughness	Organic	USCS Group	Primary
Inorganic	Group	туре	01 301	Tests	Dilatancy	Dry Strength	Shine Test	Thread Diameter	(of 3 mm thread)	Content	Symbol	Name
				Liquid Limit	Rapid	None	None	>6 mm	roll 3 mm thread)	<5%	ML	SILT
(ss)	75 mm	S	icity low)	<50	Slow	None to Low	Dull	3mm to 6 mm	None to low	<5%	ML	CLAYEY SILT
by me	OILS an 0.0	SILTS tic or P	n Plast n Plast nart be		Slow to very slow	Low to medium	Dull to slight	3mm to 6 mm	Low	5% to 30%	OL	ORGANIC SILT
GANIC t ≤30%	NED S	-Plac		Liquid Limit	Slow to very slow	Low to medium	Slight	3mm to 6 mm	Low to medium	<5%	МН	CLAYEY SILT
INOR	E-GRAI	SN)		≥50	None	Medium to high	Dull to slight	1 mm to 3 mm	Medium to high	5% to 30%	ОН	ORGANIC SILT
rganic	FINE by mas	plot e on		Liquid Limit <30	None	Low to medium	Slight to shiny	~ 3 mm	Low to medium	0% to	CL	SILTY CLAY
0	≥50%	CLAYS	e A-Lir ticity C below)	Liquid Limit 30 to 50	None	Medium to high	Slight to shiny	1 mm to 3 mm	Medium	30%	CI	SILTY CLAY
		C (Pl a above Plast		Liquid Limit ≥50	None	High	Shiny	<1 mm	High	(see Note 2)	СН	CLAY
×S	nic .30% ss)	Peat and mix	mineral soil tures							30% to 75%		SILTY PEAT, SANDY PEAT
HIGHL DRGAN SOIL	(Organ ntent > by mas	Predomir may con	nantly peat, Itain some							75%	PT	
40	ပိ	mineral so amorph	il, fibrous or nous peat							100%		PEAT
-	Low	Plasticity		Medium Plasticity	≺ Hig	h Plasticity		a hyphen,	<b>bol</b> — A dua for example,	GP-GM, S	two symbols : SW-SC and Cl	separated by ML.
					CLAY	Bud Tallit		For non-co	hesive soils,	the dual s	ymbols must b	e used when
30 -					СН			the soil h	as between I material b	5% and <sup>•</sup> etween "c	12% fines (i.e lean" and "di	e. to identify rtv" sand or
								gravel.				lity cana ci
idex (PI				CI	CLAYEY SI ORGANIC S	BILT OH		For cohes	ive soils, the	he dual symbol must be used when the		
- 02 In				ime				of the plas	and plasticity	/ Index val ee Plastici	ues plot in the itv Chart at left	CL-IVIL area
Plas		SILTY O		*							,	,
10		CL						Borderlin	e Symbol —	A borderl	ine symbol is	two symbols
7			C OF	LAYEY SILT <b>ML</b> RGANIC SILT <b>OL</b>				A borderlin	ne symbol sh	ould be us	sed to indicate	that the soil
4	has been identified as having properties that are on the					are on the						
0	SILT ML (	See Note 1)						transition b	between simil	ar materia	ls. In addition	a borderline
o	10	20	25.5 30 Li	40 5 quid Limit (LL)	<b>0</b> 60	70	80	symbol ma within a st	ay be used to ratum	indicate a	a range of simi	iar soil types
Note 1 – Fi slight plas	lote 1 – Fine grained materials with PI and LL that plot in this area are named (ML) SILT with light plasticity. Fine-grained materials which are non-plastic (i.e. a PL cannot be measured) are											

#### The Golder Associates Ltd. Soil Classification System is based on the Unified Soil Classification System (USCS)

named SILT. Note 2 – For soils with <5% organic content, include the descriptor "trace organics" for soils with between 5% and 30% organic content include the prefix "organic" before the Primary name.

#### ABBREVIATIONS AND TERMS USED ON RECORDS OF BOREHOLES AND TEST PITS

#### PARTICI E SIZES OF CONSTITUENTS

Soil Constituent	Particle Size Description	Millimetres	Inches (US Std. Sieve Size)
BOULDERS	Not Applicable	>300	>12
COBBLES	Not Applicable	75 to 300	3 to 12
GRAVEL	Coarse Fine	19 to 75 4.75 to 19	0.75 to 3 (4) to 0.75
SAND	Coarse Medium Fine	2.00 to 4.75 0.425 to 2.00 0.075 to 0.425	(10) to (4) (40) to (10) (200) to (40)
SILT/CLAY	Classified by plasticity	<0.075	< (200)

#### MODIFIERS FOR SECONDARY AND MINOR CONSTITUENTS

Percentage by Mass	Modifier
>35	Use 'and' to combine major constituents ( <i>i.e.</i> , SAND and GRAVEL)
> 12 to 35	Primary soil name prefixed with "gravelly, sandy, SILTY, CLAYEY" as applicable
> 5 to 12	some
≤ 5	trace

#### PENETRATION RESISTANCE

#### Standard Penetration Resistance (SPT), N:

The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) required to drive a 50 mm (2 in.) split-spoon sampler for a distance of 300 mm (12 in.). Values reported are as recorded in the field and are uncorrected.

#### **Cone Penetration Test (CPT)**

An electronic cone penetrometer with a 60° conical tip and a project end area of 10 cm<sup>2</sup> pushed through ground at a penetration rate of 2 cm/s. Measurements of tip resistance (q), porewater pressure (u) and sleeve frictions are recorded electronically at 25 mm penetration intervals.

Dynamic Cone Penetration Resistance (DCPT); Nd: The number of blows by a 63.5 kg (140 lb) hammer dropped 760 mm (30 in.) to drive uncased a 50 mm (2 in.) diameter, 60° cone attached to "A" size drill rods for a distance of 300 mm (12 in.).

- PH: Sampler advanced by hydraulic pressure
- PM: Sampler advanced by manual pressure
- WH: Sampler advanced by static weight of hammer
- WR: Sampler advanced by weight of sampler and rod

Compactness <sup>2</sup>				
Term	SPT 'N' (blows/0.3m) <sup>1</sup>			
Very Loose	0 to 4			
Loose	4 to 10			
Compact	10 to 30			
Dense	30 to 50			
Very Dense	>50			

NON-COHESIVE (COHESIONLESS) SOILS

- 1. SPT 'N' in accordance with ASTM D1586, uncorrected for the effects of overburden pressure.
- Definition of compactness terms are based on SPT 'N' ranges as provided in Terzaghi, Peck and Mesri (1996). Many factors affect the recorded SPT 'N' 2. value, including hammer efficiency (which may be greater than 60% in automatic trip hammers), overburden pressure, groundwater conditions, and grainsize. As such, the recorded SPT 'N' value(s) should be considered only an approximate guide to the soil compactness. These factors need to be considered when evaluating the results, and the stated compactness terms should not be relied upon for design or construction.

Field Moisture Condition

Term	Description
Dry	Soil flows freely through fingers.
Moist	Soils are darker than in the dry condition and may feel cool.
Wet	As moist, but with free water forming on hands when handled.

SAMPLES	
AS	Auger sample
BS	Block sample
CS	Chunk sample
DD	Diamond Drilling
DO or DP	Seamless open ended, driven or pushed tube sampler – note size
DS	Denison type sample
GS	Grab Sample
MC	Modified California Samples
MS	Modified Shelby (for frozen soil)
RC	Rock core
SC	Soil core
SS	Split spoon sampler – note size
ST	Slotted tube
ТО	Thin-walled, open - note size (Shelby tube)
TP	Thin-walled, piston - note size (Shelby tube)
WS	Wash sample

#### SOIL TESTS

w	water content
PL, w <sub>p</sub>	plastic limit
LL, wL	liquid limit
С	consolidation (oedometer) test
CHEM	chemical analysis (refer to text)
CID	consolidated isotropically drained triaxial test1
CIU	consolidated isotropically undrained triaxial test with porewater pressure measurement <sup>1</sup>
D <sub>R</sub>	relative density (specific gravity, Gs)
DS	direct shear test
GS	specific gravity
М	sieve analysis for particle size
MH	combined sieve and hydrometer (H) analysis
MPC	Modified Proctor compaction test
SPC	Standard Proctor compaction test
OC	organic content test
SO <sub>4</sub>	concentration of water-soluble sulphates
UC	unconfined compression test
UU	unconsolidated undrained triaxial test
V (FV)	field vane (LV-laboratory vane test)
γ	unit weight

Tests anisotropically consolidated prior to shear are shown as CAD, CAU. 1.

	COHESIVE SOILS	
	Consistency	
Term	Undrained Shear Strength (kPa)	SPT 'N' <sup>1,2</sup> (blows/0.3m)
Very Soft	<12	0 to 2
Soft	12 to 25	2 to 4
Firm	25 to 50	4 to 8
Stiff	50 to 100	8 to 15
Very Stiff	100 to 200	15 to 30
Hard	>200	>30

1. SPT 'N' in accordance with ASTM D1586, uncorrected for overburden pressure effects; approximate only.

SPT 'N' values should be considered ONLY an approximate guide to consistency; for sensitive clays (e.g., Champlain Sea clays), the N-value approximation for consistency terms does NOT apply. Rely on direct 2 measurement of undrained shear strength or other manual observations.

	Water Content
Term	Description
w < PL	Material is estimated to be drier than the Plastic Limit.
w ~ PL	Material is estimated to be close to the Plastic Limit.
w > PL	Material is estimated to be wetter than the Plastic Limit.

Unless otherwise stated, the symbols employed in the report are as follows:

I.	GENERAL	(a) w	Index Properties (continued)
π	3.1416	w <sub>l</sub> or LL	liquid limit
ln x	natural logarithm of x	w <sub>p</sub> or PL	plastic limit
log <sub>10</sub>	x or log x, logarithm of x to base 10	Ip OF PI	plasticity index = $(W_l - W_p)$
y t	time		shrinkage limit
		IL	liquidity index = $(w - w_p) / I_p$
		lc	consistency index = $(w_l - w) / I_p$
		emax	void ratio in loosest state
		emin	void ratio in densest state
II.	STRESS AND STRAIN	ID	(formerly relative density) $(e_{max} - e_{min})$
	aboar atrain	(b)	Hydroulia Proportion
Ŷ	shear sharin	(D) b	hydraulic head or potential
Δ S	linear strain	a a	rate of flow
e Ev	volumetric strain	ч V	velocity of flow
n	coefficient of viscosity	i	hydraulic gradient
υ	Poisson's ratio	k	hydraulic conductivity
σ	total stress		(coefficient of permeability)
$\sigma'$	effective stress ( $\sigma' = \sigma - u$ )	j	seepage force per unit volume
$\sigma'_{vo}$	initial effective overburden stress		
σ1, σ2, σ3	principal stress (major, intermediate,	(c)	Consolidation (one-dimensional)
	1111101)	(C) Co	compression index
Ooct	mean stress or octahedral stress	Ct	(normally consolidated range)
0001	$= (\sigma_1 + \sigma_2 + \sigma_3)/3$	Cr	recompression index
τ	shear stress		(over-consolidated range)
u	porewater pressure	Cs	swelling index
E	modulus of deformation	Cα	secondary compression index
G	shear modulus of deformation	mv	coefficient of volume change
ĸ	bulk modulus of compressibility	Cv	direction)
		Ch	direction)
		Tv	time factor (vertical direction)
III.	SOIL PROPERTIES	U	degree of consolidation
(2)	Index Properties	σ΄ρ	pre-consolidation stress
(a)	hulk density (bulk unit weight)*	UCK	over-consolidation ratio = $\sigma_p / \sigma_{vo}$
$D_{4}(\lambda^{4})$	dry density (dry unit weight)	(d)	Shear Strength
$\rho_{u}(\gamma_{w})$	density (unit weight) of water	τρ. τr	peak and residual shear strength
ρ(γs)	density (unit weight) of solid particles	φ'	effective angle of internal friction
γ'	unit weight of submerged soil	δ	angle of interface friction
	$(\gamma' = \gamma - \gamma_w)$	μ	coefficient of friction = tan $\delta$
D <sub>R</sub>	relative density (specific gravity) of solid	C'	effective cohesion
-	particles ( $D_R = \rho_s / \rho_w$ ) (formerly $G_s$ )	Cu, Su	undrained shear strength ( $\phi = 0$ analysis)
e		p n/	mean total stress $(\sigma_1 + \sigma_3)/2$
S	degree of saturation	p D	$(\sigma_1 - \sigma_2)/2$ or $(\sigma_1 - \sigma_2)/2$
0		Ч Qu	compressive strength ( $\sigma_1 - \sigma_3$ )
		St	sensitivity
* Donoi	ty symbol is a Unit weight symbol is	Notes: 1	$r = c' + c' \tan \phi'$
where	$\gamma = \rho q$ (i.e. mass density multiplied by	2	shear strength = (compressive strength)/2
accele	eration due to gravity)		(

PF	10	JEC	T No.: 20143647 / 2000		F	RE	co	R	0 0	F BC	REH	IOLE	: BH2	20-01							S		1 OF 1	
PF		NT. JECT ATIO	CF Rail T: Cascadia Phase 4 Viterra East N: Mile 123.89									TE: Jun	ie 25, 202	20 Potech D	rilling	n Son	vices	l td			D	ATUN		5
N: No	~5	4598 Iorthin	386 E: ~499380 Ig and Easting Coordinates have been determined by				INIC		AT10		,	NIKAC	TOR. GE	OLECHID	1 1111 10	9 361	nces	LIU.	SA	AMPL	_ER I	HAMM	ER, 64kg; DROP,	762mm
ш		Ģ	SOIL PROFILE				SAM	PLE	S	W30	ATER C		PERCE	NT	CL		DATI TICLE S	ON % IZE <= 0	.002	% X	4T %	. (7)		ER,
SCAL	NG RIG	MET		LOT		н		۲۶ %	.3m	Wp 1	0 :	20 3	NP - Nor 30 4	NI n-Plastic 0	Ш		s			≺ INDE	CONTEN	TONAL	OR	OR
DEPTH			DESCRIPTION	RATA F	DEPTH	NUMBE	TYPE	COVEF	O/S/(0	SHEAF Cu, kPa	R STREI	NGTH r r	nat V. + rem V.⊕	Q - ● U - ●	GRAV	SAN	FINE	SILT	CLA	ASTICIT	GANIC 0	ADDIT AB. TI	INSTALLAT	ION
		Щ		STF	(m)	-		ШЙ	Ш	2	0 ·	40 E	50 8	0						7	ORG	_		
- 0			Ground Surface FILL - (GP) GRAVEL, coarse,	$\otimes$	0.00	-																		-
Ē			angular; grey; dry.		>																		Gravel	
E			FILL - (SM) SILTY SAND, fine to	×	0.61																			
- 1			angular to sub-angular gravel; brown; grey; moist, loose to compact.		>	1	SS	50	24															
-																								
E																								
-	Ē				>	2A	ss	58	7															-
	ounted [		FILL - (ML) sandy SILT, low plasticity, fine to coarse sand; brown, with organics: w>PL inferred soft	$\bigotimes$	2.13	2B						но								2				
E	rack M	r Rotary	FILL - (SM) SILTY SAND, fine to coarse sand, trace fine to coarse		> >																			
	te Mito 7	Ä	gravel; greyish brown; moist, dense to very dense.		*																			
- 3	Fras				×	34		-															Bentonite Chips	
E			INFERRED BEDROCK - (SM) SAND	$\bigotimes$	3.35	38	ss	92	84															
			and SIL I, fine to coarse sand; grey; dry, very dense.																					
- 4																								
E						4	GS				С													
E																								
- 5						5	SS	132	>100	0					0	51	49	37	12					
Ē			End of Borehole at target depth.		4.98																			-
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DE	EPT	TH S	cALE spin-spuor sampling and recorded blow within air-rotary or sonic cored borehole, a me strictly comply with ASTM standards. Caution therefore be exercised in the intermretation of t	thod wh and jud	were comp nich does n dgment sho asured blow	ot ould v				G	01	D	ER	JUIL	ULA	JJIFI	GATI	UN S	LO	GGE	D: BE	3		0
1 1008N	: 5	0	counts and their correlation with standard "N"	values.	.cureu DION			V										C	HEC	KED	: JDS	3		U

	PRO	OJE(	CT No.: 20143647 / 2000		F	REC	co	RD	0	F BC	REH	IOLE	: BH2	20-02							SI	HEET	1 OF 1	
			CP Rail CT: Cascadia Phase 4 Viterra East DN: Mile 123.75							DRILL		TE: Ju	ne 27, 20	20 Potech D	rilling	Sen	vices	l td			D	ATUM	Ground Sunace	
	N: ~ Note. GPS	5459 North	0772 E: ~499538 ing and Easting Coordinates have been determined by field and are approximate only.				INC	CLIN	ATIO	DINIEL	°		FIOR. Ge	OLECHIL	/	JOEN	lices	LIU.	SA	AMPL	ER H	HAMM	ER, 64kg; DROP,	762mm
щ	Ī	υQ	SOIL PROFILE			:	SAM	PLE	S	W	ATER C	ONTEN		NT	CL/	GRA	DATI TICLE S	ON % IZE <= 0	.002	% X	NT %	_ 0	PIEZOMETE STANDPIP	ER, E
DEPTH SCAL	MEIKES	DRILLING RI	DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	түре	RECOVERY %	BLOWS/0.3m	WI 1 SHEAI Cu, kP 2	ρ 2 R STREM a	<u>ар</u> идтн Ю	NP - Nor 30 4 nat V. + rem V. ⊕ Pocket F 60 8	VI 1-Plastic 0 Q - ● U - ● Pen - ■ 0	GRAVEL	SAND	FINES	SILT	CLAY	PLASTICITY INDE	ORGANIC CONTE	ADDITIONAI LAB. TESTIN	OR THERMISTO INSTALLATI	OR ON
_	0		Ground Surface FILL - (GP) GRAVEL, coarse.	$\overline{\mathbf{x}}$	> 0.00																			KO8081
			FILL - (SM) gravelly SILTY SAND, fine to coarse sand, fine to coarse		0.61																		Gravel	
	1		brown, with iron oxide staining; moist, loose to compact.			1	SS	63	8						19	56	25							-
	2				2.13	2	SS	71	12		0													- - - - - - - - - - - - - - - - - - -
	3	< Mounted Drill tarv	fine to coarse sand, fine sub-angular gravel; brownish grey; moist, loose to compact.																					
		aste Mito Track Air Ro				3	SS GS	0	3		0-	-1			20	52	28			7			Bentonite Chips	
-	4	Ē	- Log or root encountered from 4.1 to 4.3 m.		4.27	5A 5B	SS	46	32		0				2	50	48	36	12					-
	5		BEDROCK - <b>(SM)</b> SAND and SILT, fine to medium sand, trace gravel; grey; moist, dense.			6	SS	92	41															
	6		INFERRED BEDROCK - (SM) SAND and SILT, fine to coarse sand; grey; dry, very dense.		5.49																			
						7	SS	105	>100		0				0	58	42	32	10					-
	7 8 9		End of Borehole at target depth.		6.55	Dieted								SOIL	CLA	SSIFI	CATI	ON S	YSTE	EM: G	GACS	6		
	DEF 1 :	РТН 50	SCALE strictly comply with ASTM standards. Caution therefore be exercised in the interpretation of counts and their correlation with standard "N"	the mea values.	dgment sho asured blow	ould v				G	OL	. D	ER					C	LOO HEC	ggei Ked	D: BE : JDS	3 6		0

PROJECT No.: 20143647 / 2000
CLIENT: CP Rail
FROJECT. Cascalia Fliase 4 Vilei

#### **RECORD OF BOREHOLE: BH20-03**

SHEET 1 OF 1 DATUM: Ground Surface

erra East LOCATION: Mile 123.62 N: 5459725.00 E: 499767.00 GPS

DRILLING DATE: June 27, 2020 DRILLING CONTRACTOR: Geotech Drilling Services Ltd.

SAMPLER HAMMER, 64kg; DROP, 762mm

-	Τ	6								DN: -90 W	。 ATER C	ONTEN	T PERCE	NT		GRA		ON %		%	%		PIEZOMETER,
ALE	0	BIG		F	1	-		LES	ر ح	Wp		0W		WI - Plactic	CL	AY PAR	TICLE S	IZE <= 0	0.002	IDEX :	TENT	'ING	STANDPIPE
TH SC		5NG		A PLO	ELEV.	BER	붠	ERY 9	S/0.3rr	1 SHEAF	ρ ź R STREI	2ρ NGTH	_NP-No 30 4 natV. +	Q - ●	VEL	QN	AES 1	5	Α	CITY IN	C CON	TEST	THERMISTOR
DEPT	Z			'RAT/	DEPTH (m)	NUM	Σ	COV	LOWS	Cu, kP	a		rem V. Pocket	Ū - Ŭ Pen - ■	GRA	SA	L L	S	Ы	ASTIC	GANIC	ADC LAB.	
	_	Ĭ		ST	(11)			R	B	2	0 4	40	60 8	80 I						₫	ő		
-	0		Ground Surface FILL - (GP) GRAVEL, coarse,	$\sim$	5.80																		00000
F			angular; grey; dry.																				
F					5.19																		Gravel
E			FILL - (ML) CLAYEY SILT, some fine	Ŵ	0.61																		
F	1		sub-angular to angular gravel;		Š																		
F			firm to stiff.		Ś	1	SS	67	7											19			
E					ž																		
F				$\otimes$	8																		
F					Š	2	SS	46	11			0											
E	2	ed Dri																					Water level
-		Nount	<u> </u>	$\otimes$	3.36																		observed in
-		Lack	평 INFERRED WEATHERED 언 BEDROCK - (ML) SILT and SAND,		2.44																		during drilling
E		, Mito	fine to coarse sand; grey, with iron oxide staining; dry, dense.																				
-	3	Fraste																					Bentonite Chips
E					2.45	ЗA	ss	92	39		0				0	42	58	44	14				
E			and SILT, fine to coarse sand; grey;		3.35	ЗB																	
-			dry, very dense.		1																		
E	4			$\mathbb{N}$																			
F																							
F					]																		
E				$\otimes$																			
F	5			$\mathbb{K}$		4	SS	0	>100														
E	ŀ		End of Borehole at target depth.	-122	5.13																		3222
F																							
-																							
E	6																						-
F																							
Ē																							
11/19/2																							
oquila T																							
SL 810	7																						-
UT0)2																							
SRADA																							
- 19	8																						-
BORE																							
o'L L																							
utput F.																							
et D: C	9																						-
ue Proje																							
IN Unic																							
TIONAL																							
	10																						-
GINT_C																							
Server.			* Note: Split-Spoon sampling and recorded blow within air-rotary or sonic cored borehole, a me	v counts ethod wh	s were comp hịch does n	oleted ot				~	<u>~ ·</u>			SOIL	CLA	SSIFI	ICATI	ON S	YSTE	EM: C	GACS	;	REV:
tional IN	∪⊏⊦ 1 ·	50	Strictly comply with AS IM standards. Caution therefore be exercised in the interpretation of counts and their correlation with standard "N"	n and jud the mea values.	agment sho asured blow	ouid /				G		ט.	ER	ł.				~		GE	D: BB	5	0
Nai	۰.	50																(	-HEC	NED	. 108	)	

# PROJECT No.: 20143647 / 2000

#### **RECORD OF BOREHOLE: BH20-04**

SHEET 1 OF 2 DATUM: Ground Surface

CLIENT: CP Rail PROJECT: Cascadia Phase 4 Viterra East LOCATION: Mile 123.47 N: 5459741.00 E: 499982.00 GPS

DRILLING DATE: June 28, 29, and July 7 2020 DRILLING CONTRACTOR: Geotech Drilling Services Ltd.

SAMPLER HAMMER, 64kg; DROP, 762mm

		1			-	INC	;LIN/		JN: -90°										-	-	
щ	υĘ	SOIL PROFILE			5	SAM	PLES	3	WATER C	ONTEN	T PERCE	NT	CL	GRA AY PAR	DATI TICLE S	ON % IZE <= 0	0.002	% X	NT %	ں ۔	PIEZOMETER, STANDPIPE
DEPTH SCA METRES	DRILLING RI	DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	RECOVERY %	BLOWS/0.3m	10 2 SHEAR STREM Cu, kPa 20 4	<u>20</u> NGTH 40	NP - No 30 4 nat V. + rem V. ⊕ Pocket 60 8	Q - ● U - ● Pen - ■ 80	GRAVEL	SAND	FINES	SILT	CLAY	PLASTICITY INDE	ORGANIC CONTE	ADDITIONA LAB. TESTIN	OR THERMISTOR INSTALLATION
		Ground Surface		6.00																	
- 0 - - - - -		FILL - (GP) GRAVEL, coarse, angular; grey; dry.		0.00 5.39	-																Gravel
- - - 1 -		fine to coarse sub-angular to angular gravel, fine to coarse sand; brownish grey; dry, compact.			1	ss	38	20													
		FILL - (MH) CLAYEY SILT, high plasticity: arey, with iron oxide staining		<u>4.17</u> 1.83	2A 2B	ss	67	7		0	 	51	1					19			
		and coal fragments up to 2 mm; w <pl, firm. FILL - (SC) gravelly SAND and CLAYEY SILT, fine to coarse sand, transfirm to coarse outbraveliat to</pl, 		3.56 2.44																	
- - - - -		angular gravel; brownish grey, with organics; w~PL, stiff. - log encountered from 3.0 to 3.7 m.			3	SS	25	45													
- - - - - 4		- Driller notes no drilling resistance and possible void from 3.7 to 4.1 m.			4	GS															
- - - - - 5	ack Mounted Drill Rotary	(ML) SILT to CLAYEY SILT; grey; wet, compact.		1.43 4.57	5	SS	55	23		н	0							1			Water level observed in open hole during drilling
	Fraste Mito Tr Air																				Bentonite Chips
				-0.71	6	SS	100	22		0			0	0	100	80	20				
		(CI, CL and ML) CLAYEY SILT and SILTY CLAY, low to medium plasticity; grey, with 1-10 mm thick light grey laminations; w>PL, very stiff.		6.71																	
- - - - -					7	ss	100	33			ъч		0	1	99	54	45	15			
- 9																					
					8	SS	100	27			0										
- 10	┝└			1	┝┥		-			+	-	+			┣		+ -		+-	<u> </u>	
┝──	I	* Note: Solit Soors complian and recorded bla			later							<u>م</u>								Ļ	RFV <sup>.</sup>
DE 1	ертн : 50	SCALE within air-rotary or sonic cored borehole, a me strictly comply with ASTM standards. Caution therefore be exercised in the interpretation of counts and their correlation with standard "N"	thod wh and juo the mea values.	ich does n dgment sho sured blow	ot ould				GOL	D	ER	JUIL	ULA	ויזוטע	5411	(	LO	GGEI CKED	D: BE	, 3 3	0

PI		ECT No.: 20143647 / 2000 IT: CP Rail		R	EC	DR	RD (	OF B		IOLE	: BH2	20-04							SI D/	HEET	2 OF 2 Ground Surface	9
LC N:	CAT 5459	TION: Mile 123.47 19741.00 E: 499982.00 GPS				101		DRILI	LING DA	NTRAC	ie 28, 29 TOR: G	9, and Ju eotech D	iy 72 Prilling	g Serv	ices	Ltd.	SA	AMPL	.ER F	HAMM	ER, 64kg; DROP,	762mm
ц	Ð	SOIL PROFILE			SA		.INA I .ES	<u>V</u>	U <sup>°</sup> VATER C /n I		PERCE	NT	CLA	GRA AY PAR	DATIO	ON % IZE <= 0	.002	EX %	ENT %	2 G	PIEZOMET STANDPIF	ER, PE
DEPTH SCA METRES	DRILLING		STRATA PLOT (u) BTRATA PLOT	V. TH )	NUMBER		BLOWS/0.3m	SHEA Cu, kl	10 NR STREI Pa 20	20 ( NGTH 1 40 (	NP - No 30 nat V. + rem V. ⊕ Pocket 50 {	n-Plastic 40 Q - ● 0 U - ● Pen - ■ 80	GRAVEL	SAND	FINES	SILT	CLAY	PLASTICITY IND	ORGANIC CONTI	ADDITION/ LAB. TESTII	OR THERMIST INSTALLAT	OR ION
	Fraste Mto Track Mounted Drill DR	Image: Provide a stars         Image: Provide a stars<		3.80	□N ⊢ 9 S 10 S		ADT         ADT           ADT						0		99	76	23	2 2			Bentonite Chips	
    20																						
DI 1	EPTH : 50	* Note: Split-Spoon sampling and recorded blow within air-rotary or sonic cored borehole, a re H SCALE strictly comply with ASTM standards. Caulion therefore be exercised in the interpretation of counts and their correlation with standard "N"	counts were of thod which do and judgment he measured values.	omplei es not shoul blow	ted Id			G	οι	D	ER	SOIL	CLAS	SSIFI	CATI	ON S	YSTE LOO	EM: ( GGE	GACS D: BE	5 3 3		REV: 0

PROJECT No.: 20143647 / 2000

#### **RECORD OF BOREHOLE: BH20-05**

SHEET 1 OF 2 DATUM: Ground Surface

CLIENT: CP Rail PROJECT: Cascadia Phase 4 Viterra East LOCATION: Mile 123.35 N: 5459831.00 E: 500131.00 GPS

DRILLING DATE: July 3, 2020 DRILLING CONTRACTOR: Geotech Drilling Services Ltd.

SAMPLER HAMMER, 64kg; DROP, 762mm

						-	IN	CLIN	IATI	ON: -9	0°			050			05	D 4 7	011.01		т. Т	1.	1		
9			SOIL PROFILE				SAN	IPLE	s	V	vATER	CONT	ENT	PERCE	NT WI	CL	GRA AY PAR	DATI TICLE S	UN % IZE <= 0	.002	EX %	ENT %	μģ	PIEZOMET	ΞR, Έ
		Ĭ		LOT		н		۲%	).3m		10	20	3	NP - No	n-Plastic	H	6	s	.	~	ND	CONTE	10N/	OR THERMIST	OR
			DESCRIPTION	VTA F	ELEV.	JMBE	TYPE	3VEF	WS/C	SHEA Cu, k	√R STR Pa	RENGT	l n	at V. + em V. ⊕	Q - ● U - ●	RAV	SAN	FINE	SILT	CLAN	STICIT	NIC C	B. TE	INSTALLATI	ON
ľ				STR/	(m)	ľ	[	REC	BLO		20	40	6	Pocket 0 8	Pen - 📕	0		-			PLAS	ORGA	LA A		
t		╈	Ground Surface		6.20	,															$\square$				
			FILL - (GP) GRAVEL, coarse,	$\otimes$	0.00	)																			
			angular, groy, ary.		Š																				
		L			5.59	,																		Gravel	
			FILL - (SM) gravelly SILTY SAND, fine to coarse sand, fine to coarse		0.61																				
			sub-angular to angular gravel; brown;	$\otimes$	8																				Ň
			ary, compact.		Ś																				25
					8																				3
					Š																				200
				$\otimes$	3	1	SS	33	16																Ş
					4.07	,																			
			FILL - (CL) sandy SILTY CLAY, fine	Ŵ	2.13	5																			322
			sub-angular to angular gravel;	$\otimes$	X																				22
			brownish grey, with wood debris; w>PL, stiff.	$\otimes$	Š																				32.5
					3																				3.3
				$\otimes$	8	$\vdash$				-															2.2
						2	ss	13	3		0														2.2
					X	-																			222
					3																				3
					3																				2.2
					1.93	3A	SS	54	8					0										Water level	2020
=			(CI, CL and ML) CLAYEY SILT and	Ź	4.27	3B							0											open hole	
			grey, with 1-10 mm thick light grey				-		-	-														during drilling	100
Annual		2	laminations; w>PL; w>PL, stiff to hard.					50	11			h													200
Asland		KOTAL				4	SS	50	11			β													254
T of		₹				$\vdash$	+			-															1949
A of of																								Bentonite Chips	3.3
Ľ	Ē				1																				202
																									2023
						5	SS	100	14			-	0								5				22.2
						$\vdash$	-		-																22.52
																									222
																									2.12
																									2.22
																									2023
																									2.5
						6	SS	100	22				0												2.2
					1																				22
																							1		
																									and a
1																									100
					ł					1															12.22
																									2.2
						7	SS	100	32				(	þ											2.02
																									2.2
		-	CONTINUED NEXT PAGE		T	Γ	Γ	1	Γ.	Γ	1	- T -			Γ			<b>–</b> –				T			
-			* Note: Split-Spoon sampling and recorded blow	counts	were comp	pleted								I	SOIL	CLA	SSIFI	CATI	ON S	YSTI	EM:	GACS	3		RE
ΞP	ΤH	SC	CALE strictly comply with ASTM standards. Caution therefore be exercised in the interpretation of t	and jud the mea	dgment sho asured blow	ould v		I		G	0	LC	)	ER	2					LO	GGE	ED: BE	З		
:	50		counts and their correlation with standard "N"	values.			~	V		-									C	HEC	CKED	D: JDS	S		
						_	_																		

PF CI PF		ECT	No.: 20143647 / 2000 CP Rail : Cascadia Phase 4 Viterra East - Mile 123 35		F	RE	co	RD	0	D <b>F BC</b>	<b>DREH</b>	OLE	<b>BH2</b> y 3, 2020	20-05							SI D/	HEET ATUM:	2 OF 2 Ground Surface	9
N:	545	5983	1.00 E: 500131.00 GPS				INC	CLIN	ATIC	DRILL DN: -90	NG COI °	NTRAC	TOR: Ge	eotech D	rilling	Serv	vices	Ltd.	SA	AMPL	ER F	AMM	ER, 64kg; DROP,	762mm
ALE	RG	THOD	SOIL PROFILE				SAM	PLES	~	w W	ATER CO	INTENT OW		NT WI	CL/	GRA	DATIO	ON % IZE <= 0	.002	DEX %	'ENT %	AL NG	PIEZOMET STANDPIF	ER, PE
DEPTH SC METRES	DRILLING	DRILLING ME	DESCRIPTION	STRATA PLO	ELEV. DEPTH (m)	NUMBER	TYPE	RECOVERY %	BLOWS/0.3m	1 SHEAI Cu, kP 2	ρ 2 R STREN a 0 4	р : IGTH 0 (	NP - Nor 30 4 nat V. + rem V. ⊕ Pocket F 60 8	Q - ● U - ● Pen - ■ 0	GRAVEL	SAND	FINES	SILT	CLAY	PLASTICITY IN	ORGANIC CONI	ADDITION LAB. TEST	THERMIST	or Ion
- 10 - - - - -		Air Rotary	(CI, CL and ML) CLAYEY SILT and SILTY CLAY, low to medium plasticity; grey, with 1-10 mm thick light grey laminations; w>PL; w>PL, stiff to hard. (continued) End of Borehole at target depth.		<u>-4.47</u> 10.67																		Bentonite Chips	
			* Note: Split-Spoon sampling and recorded blow	counts	were comp	bleted								SOIL	CLAS	SSIFI	CATIO	ON S	YSTE	EM: 0	GACS			
	EPTI : 50	H SC	CALE strictly comply with ASTM standards. Caution therefore be exercised in the interpretation of counts and their correlation with standard".	noa wh and jud he mea values.	ucn does n dgment sho asured blow	ot ould '		Ç		G	OL	D	ER					C	LO( HEC	GGEI KED	D: BE : JDS	3		0

PROJECT No.: 20143647 / 2000 CLIENT: CP Rail PROJECT: Cascadia Phase 4 Viterra East LOCATION: Mile 123.71

#### **RECORD OF BOREHOLE: BH20-06**

SHEET 1 OF 1 DATUM: Ground Surface

DRILLING DATE: July 5, 2020 DRILLING CONTRACTOR: Geotech Drilling Services Ltd.

				IN	CLIN	ΑΤΙΟ	ON: -90°								SA		ER F	IAMM	≟R, 64kg; DROP, 7		
SOIL PROFILE					IPLE:	S	WAT	ER CONT		NT	CL	GRA AY PAR	DATI	ON %	0.002	EX %	ENT %	- <sup>O</sup>	PIEZOMETE STANDPIPE		
DESCRIPTION	STRATA PLOT	ELEV DEPTH (m)	NUMBER	TYPE	RECOVERY %	BLOWS/0.3m	MP F 1Ω SHEAR S Cu, kPa 20	20 STRENGTI 40	NP - No 30 4 H nat V. + rem V. ⊕ Pocket I 60 8	Q - ● U - ● Pen - ■	GRAVEL	SAND	FINES	SILT	CLAY	PLASTICITY IND	DRGANIC CONTE	ADDITIONA LAB. TESTIN	OR THERMISTO INSTALLATIC		
Ground Surface FILL - (GP) GRAVEL, coarse, angular; grey; dry. (ML) sandy CLAYEY SILT, mediu to high plasticity, fine to coarse sa		5.6 0.0 4.9 0.6	0 0 9 11																Gravel		
somě fine to coarse sub-angular t angular gravel; greyish brown, wit iron oxide staining; w <pl td="" to="" w~pl<=""><td></td><td></td><td>1</td><td>ss</td><td>58</td><td>11</td><td></td><td>0</td><td></td><td>1</td><td></td><td></td><td></td><td></td><td></td><td>19</td><td></td><td></td><td></td></pl>			1	ss	58	11		0		1						19					
- log or root encountered from 3.7 3.8 m.	r to		2	ss	42	11		С											Water level		
(GP) sandy GRAVEL, fine to coar sand, fine to coarse sub-angular t angular gravel; grey; wet.		1.3 4.2	<u>3</u> 7	SS	68	_>100													open hole during drilling		
- Driller notes no drilling resistant and possible void from 5.3 to 6.3	e n.		4	SS	0	0					77	19	4						Bentonite Chips		
			6	SS	17	19															
INFERRED WEATHERED BEDROCK - (ML) sandy SILT, fin coarse, low plasticity; blue grey; d	e to y.	- <u>2.0</u> 7.6	2 7	ss	83	78	(	>			0	23	77	64	13						
INFERRED BEDROCK - (ML) sa SILT, fine to coarse, low plasticity grey; dry. End of Borehole at target depth.	ndy blue	-3.5 9.1 -3.7 9.3	4 7 8 7	SS	111	>100	0														
	SOIL PROFILE DESCRIPTION Ground Surface FILL - (GP) GRAVEL, coarse, angular; grey; dry. (ML) sandy CLAYEY SILT, mediu to high plasticity, fine to coarse sas some fine to coarse sub-angular ti angular gravel; grey; ext log or root encountered from 3.7 3.8 m. (GP) sandy GRAVEL, fine to coars sand, fine to coarse sub-angular te angular gravel; grey; wet Driller notes no drilling resistanc and possible void from 5.3 to 6.3 r NIFERRED WEATHERED BEDROCK - (ML) sandy SILT, fine coarse, low plasticity; blue grey; dr INFERRED BEDROCK - (ML) sarg grey; dry. End of Borehole at target depth.	SOIL PROFILE         Description           DESCRIPTION         Ground Surface           FILL - (GP) GRAVEL, coarse, angular, grey; dry.         (ML) sandy CLAYEY SILT, medium to high plasticity, fine to coarse sand, some fine to coarse sub-angular to angular gravel; greyish brown, with iron oxide staining; w <pl td="" to="" w-pl.<="">           - log or root encountered from 3.7 to 3.8 m.           (GP) sandy GRAVEL, fine to coarse sand, the to coarse sub-angular to angular gravel; grey; wet.           - log or root encountered from 3.7 to 3.8 m.           (GP) sandy GRAVEL, fine to coarse sand, fine to coarse sub-angular to angular gravel; grey; wet.           - Driller notes no drilling resistance and possible void from 5.3 to 6.3 m.           INFERRED WEATHERED BEDROCK - (ML) sandy SILT, fine to coarse, low plasticity; blue grey; dry.           INFERRED BEDROCK - (ML) sandy SILT, fine to coarse, low plasticity; blue grey; dry.</pl>	SOIL PROFILE         DESCRIPTION       Image: Colspan="2">Image: Colspan="2" Image: Col	SOIL PROFILE         DESCRIPTION       Image: Colspan="2">Image: Colspan="2" Colspan	SOIL PROFILE     SAM       DESCRIPTION     Image: Colspan="2">Image: Colspan="2">SAM       DESCRIPTION     Image: Colspan="2">Image: Colspan="2">SAM       DESCRIPTION     Image: Colspan="2">Image: Colspan="2">SAM       Colspan="2">Colspan="2">Colspan="2">Image: Colspan="2">SAM       Colspan="2">Colspan="2">Image: Colspan="2">SAM       Colspan="2">Colspan="2">Image: Colspan="2">SAM       Colspan="2">Colspan="2">Sam       FILL - (GP) GRAVEL, coarse, and, some fine to coarse sand, fine to coarse sand, fine to coarse sand, some fine to coarse sand, fine to coarse sub-angular to angular gravel; grey, wet.     1.33       IMPERNED WEATHERED     - 0.00       Implemented from 3.7 to 3.8 m.     1.33       Implemented from 3.7 to 3.8 m.     1.33       Implemented from 3.7 to coarse sand, fine to coarse sub-angular to angular gravel; grey, wet.     - 0.00       - Driller notes no drilling resistance and possible void from 5.3 to 6.3 m.     - 4.27       Implemented from 3.7 to 3.8 m.     - 2.02       Implemented from 5.3 to 6.3 m.     - 0.00       Implemented fro	SOIL PROFILE       SAMPLES         DESCRIPTION       Image: Colspan="2">SAMPLES         DESCRIPTION       Image: Colspan="2">SAMPLES         Coround Surface       Solut Profile         Ground Surface       Solut Profile         FILL - (GP) GRAVEL, coarse, angular, grey, dry.         (ML) sandy CLAYEY SILT, medium to high plasticity, fine to coarse sand, some fine to coarse sub-angular to angular gravel; grey, wet.       1 SS 58         - log or root encountered from 3.7 to 3.8 m.       1       SS 68         - log or root encountered from 3.7 to 3.8 m.       1.33       -         - Driller notes no drilling resistance and possible void from 5.3 to 6.3 m.       4       SS 0         - Driller notes no drilling resistance and possible void from 5.3 to 6.3 m.       -       -       -         INFERRED WEATHERED       -       -       -       -       -         BEDROCK - (ML) sandy SILT, fine to coarse, low plasticity, blue grey, dry.       -       -       -       -         INFERRED BEDROCK - (ML) sandy       -       -       -       -       -       -         INFERRED BEDROCK - (ML) sandy SILT, fine to coarse, low plasticity, blue grey, dry.	INCLINATION           SAMPLES           DESCRIPTION         Image: Colspan="2">SAMPLES           DESCRIPTION         Image: Colspan="2">Image: Colspan="2">SAMPLES           DESCRIPTION         Image: Colspan="2">Image: Colspan="2">SAMPLES           DESCRIPTION         Image: Colspan="2">Image: Colspan="2">SAMPLES           DESCRIPTION         Image: Colspan="2">Image: Colspan="2">SAMPLES           Colspan="2">Colspan="2">SAMPLES           Colspan="2">Colspan="2">SAMPLES           Colspan="2">Colspan="2">SAMPLES           Colspan="2">SAMPLES           Colspan="2">Colspan="2">SAMPLES           Colspan="2">Colspan="2">SAMPLES           Colspan="2">SAMPLES           Colspan="2">SAMPLES           Colspan="2">SAMPLES           Colspan="2">Colspan="2">SAMPLES           Colspan="2">Colspan="2">SAMPLES           Colspan="2">SAMPLES           Colspan="2">SAMPLES           Colspan="2">SAMPLES           Colspan= 2           Colspan= 2           Colspan= 2           Colspan= 2	NOLLATION: - 30"         WAT We h DESCRIPTION         WAT SHEARS 5 0.00         WAT We h DESCRIPTION         WAT SHEARS 5 0.00           Counce Sufface angular gravel; grey wit, grey, wet.         4.00         5.00         4.0	INCLINATION: -30" WATER CONT           SAMPLES           WATER CONT           DESCRIPTION         SAMPLES           USAMPLES           DESCRIPTION         SAMPLES           DESCRIPTION         SAMPLES           DESCRIPTION         SAMPLES           OPENATION         SAMPLES           Cound Surface         Samples           INTERCONT           INTERCONT         Samples         Samples         Samples         Samples         Samples           INTERCONT         Samples         Samples         Samples         Samples         Samples         Samples         Samples         Samples         Samples         Samples	NCLINATION - 490"           SOIL PROFILE           SAMPLES           DESCRIPTION         Solution of the second	WALLINGN: -50°           SOIL PROFILE           WALLINGN: -50°           DESCRIPTION         SAMPLES           VELOW: 100 - 0000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000 - 000	INCLARICS: -50°           SAMPLE           SAMPLE           DESCRIPTION           0         2         2         1         1         0         2         0 </td <td>INCLANCH. 300           SOL PROPILE           WATER CONTENT PERCENT           DESCRIPTION         WITE CONTENT PERCENT           CONTENT PERCENT         CONTENT PERCENT           SOLE PROPILE         CONTENT PERCENT         CONTENT PERCENT           DESCRIPTION         Sole Propine         CONTENT PERCENT         CONTENT PERCENT           DESCRIPTION         Sole Propine         CONTENT PERCENT           Sole Propine         CONTENT PERCENT           Sole Propine Provide Propine Provide Provide</td> <td>INCLIMATION: -80"         COMMENT PERCENT         COMMENT         COMMENT         C</td> <td>INCLINATION: 30"         CONVENTING PERCENT         CONVENTING PERCENT           UPERCENTION         Self-RASISTENTING         CONVENTING           CONVENTING         CONVENTING         CONVENTING         CONVENTING           CONVENTING         CONVENTING         CONVENTING         CONVENTING           CONVENTING         CONVENTING         CONVENTING         CONVENTING      CONVENTING         CONVENTING         CONVENTING         CONVENTING         CONVENTING           CONVENTING         CONVENTING         CONVENTING         CONVENTING         CONVENTING         CONVENTING         CONVENTING         <th <<="" colspan="2" td=""><td>INCLINATION - 30"           CONVENTION           <th< td=""><td>NOLLATION         COLUMINON         COLUMINON</td><td>INCLUMENT         OWNER         OWNER</td><td>INCLANTICS         OPECANTON         &lt;</td></th<></td></th></td>	INCLANCH. 300           SOL PROPILE           WATER CONTENT PERCENT           DESCRIPTION         WITE CONTENT PERCENT           CONTENT PERCENT         CONTENT PERCENT           SOLE PROPILE         CONTENT PERCENT         CONTENT PERCENT           DESCRIPTION         Sole Propine         CONTENT PERCENT         CONTENT PERCENT           DESCRIPTION         Sole Propine         CONTENT PERCENT           Sole Propine         CONTENT PERCENT           Sole Propine Provide Propine Provide	INCLIMATION: -80"         COMMENT PERCENT         COMMENT         COMMENT         C	INCLINATION: 30"         CONVENTING PERCENT         CONVENTING PERCENT           UPERCENTION         Self-RASISTENTING         CONVENTING           CONVENTING         CONVENTING         CONVENTING         CONVENTING           CONVENTING         CONVENTING         CONVENTING         CONVENTING           CONVENTING         CONVENTING         CONVENTING         CONVENTING      CONVENTING         CONVENTING         CONVENTING         CONVENTING         CONVENTING           CONVENTING         CONVENTING         CONVENTING         CONVENTING         CONVENTING         CONVENTING         CONVENTING <th <<="" colspan="2" td=""><td>INCLINATION - 30"           CONVENTION           <th< td=""><td>NOLLATION         COLUMINON         COLUMINON</td><td>INCLUMENT         OWNER         OWNER</td><td>INCLANTICS         OPECANTON         &lt;</td></th<></td></th>	<td>INCLINATION - 30"           CONVENTION           <th< td=""><td>NOLLATION         COLUMINON         COLUMINON</td><td>INCLUMENT         OWNER         OWNER</td><td>INCLANTICS         OPECANTON         &lt;</td></th<></td>		INCLINATION - 30"           CONVENTION         CONVENTION <th< td=""><td>NOLLATION         COLUMINON         COLUMINON</td><td>INCLUMENT         OWNER         OWNER</td><td>INCLANTICS         OPECANTON         &lt;</td></th<>	NOLLATION         COLUMINON         COLUMINON	INCLUMENT         OWNER         OWNER	INCLANTICS         OPECANTON         <

Pf	PROJECT No.: 20143647 / 2000         RECORD OF BOREHOLE: BH20-07         SHEET 1 OF 1																							
CLIENT: CP Kall     DATUM: Ground Surface       PROJECT: Cascadia Phase 4 Viterra East     DRILLING DATE: July 5, 2020       LOCATION: Mile 124.01     DPILLING CONTRACTOR: Control Duiling Services Ltd														e										
N: No	~5 te: N	459 Iorthir	974 E: ~499190 ng and Easting Coordinates have been determined by eld and are approximate only				INC		ΔΤΙά		, ,	NIRAC	TUR: Ge	eotech D	niiing	Serv	lices	Lta.	SA	AMPL	_ER I	HAMM	ER, 64kg; DROP	, 762mm
										W	ATER C	ONTEN		NT	CLA	GRA	DATI TICLE S	ON % IZE <= 0	0.002	% X	NT %	.0		ER, PF
DEPTH SCAL METRES	DRILING RIC	DRILLING METH	DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	<b>RECOVERY %</b>	BLOWS/0.3m	Wr 1 SHEAF Cu, kP 2	0 2 R STREM a 0 4	<u>о</u> ю	NP - Nor 30 4 nat V. + rem V. ⊕ Pocket I 60 8	VI -Plastic 0 Q - ● U - ● Pen - ■ 0	GRAVEL	SAND	FINES	SILT	CLAY	PLASTICITY INDE	ORGANIC CONTEN	ADDITIONAL LAB. TESTIN	OR THERMIST INSTALLAT	OR ION
0			Ground Surface FILL - (GP) GRAVEL, coarse.	$\sim$	0.00																-			10350351
- - - - - - - - - - -	Mounted Drill		FILL - (SP/GP) SAND and GRAVEL, fine to coarse sand, fine to coarse sub-angular to angular gravel; brown; dry.		0.61	0.00 0.61 1.52 1 SS 63 4															Gravel			
- - - - - - -		tary	FILL - (SM) gravelly SILTY SAND, fine to coarse sand, fine to coarse sub-rounded to sub-angular gravel, some plastic fines; orangish brown, with oxidation staining; moist, loose.		1.52																			
	Fraste Mito Track	Air Ro	(SM) SILTY SAND, fine to coarse sand; orangish brown, with carbonaceous fragments; moist, loose.		2.44											66	21	20					Bentonite Chips	
- - - - - - - - 4 -						2 55	63	7									20	5						
- - - - - 5			INFERRED BEDROCK - (CL) SILTY CLAY, some fine to coarse sand; orange, with oxidation staining. INFERRED BEDROCK - (ML) SILT		4.27	ЗА 3В	SS	111	80	C					0	45	55	42	13					
			and SAND, tine to coarse sand; grey; dry. End of Borehole at target depth.		5.03																			
- 7																								
																								-
9																								
- - - - - 10																								
DI 1	Ц ЕР <sup>-</sup> : 5	гн s 50	* Note: Split-Spoon sampling and recorded blow within air-rotary or sonic cored borehole, a me SCALE strictly comply with ASTM standards. Caution therefore be exercised in the interpretation of counts and their correlation with standard "N"	v counts thod wh and juo the mea values.	were comp hich does n dgment sho asured blow	oleted ot ould				G	οL	. D	ER	SOIL	CLAS	SSIFI	CATI	ON S	LO CHEC	L EM: ( GGE CKED	J GACS D: BE D: JDS	5 3 3		REV: 0

PROJECT No.: 20143647 / 2000         RECORD OF BOREHOLE: BH20-08         SHEET 1 OF 1													1 OF 1											
PROJECT: Cascadia Phase 4 Viterra East DRILLING DATE: July 8, 2020 LOCATION: Mile 122.8 DRILLING CONTRACTOP: Gentech Drilling Services Ltd													9											
	N: ~ Note	~5459 North	0810 E: ~499486 ing and Easting Coordinates have been determined by				INI	CLIN	ΙΔΤΙά	DRILL		NTRAC	TUR: Ge	eotech L	niing	j Sen	lices	Lta.	S	AMPL	ER H	HAMM	IER, 64kg; DROP	, 762mm
щ		ъ	SOIL PROFILE	SAM	PLE	s	W	ATER C	ONTEN	T PERCEI	NT	CL	GRA AY PAR	DATION TICLE S	ON % IZE <= 0	0.002	% X3	NT %	_ <u>0</u>	PIEZOMET STANDPI	ER, PE			
DEPTH SCAL	METRES	DRILLING RI	DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	TYPE	RECOVERY %	BLOWS/0.3m	W SHEA Cu, kF	ρ   <u>2</u> R STREM 'a	<u>ар</u> идтн ю	NP - Nor 30 4 nat V. + rem V. ⊕ Pocket I 60 8	WI n-Plastic 0 0 - ● U - ● Pen - ■	GRAVEL	SAND	FINES	SILT	CLAY	PLASTICITY INDE	ORGANIC CONTE	ADDITIONAL LAB. TESTIN	OR THERMIST INSTALLAT	'OR 'ION
-	0		Ground Surface FILL - (GP) GRAVEL, coarse,	$\times$	0.00	)																		102020 -
			angular; grey; dry.																				Gravel	
	1		(SM) SILTY SAND and GRAVEL, fine to coarse sand, fine to coarse sub-angular to angular gravel; grey, with inferred cobbles; dry, very dense.		0.91	1	55	75	76						38	48	14							
	2		(ML) sandy SILT, fine to coarse sand, some gravel; dark grey; dry. - becomes gravelly at 2.4 m depth.		2.13																			
	3	ck Mounted Drill otarv	(SM) gravelly SAND and SILT, fine to coarse sand, fine to coarse sub-angular to angular gravel; brownish grey; wet, dense.		3.05	2	ss	100	34		он				14	46	40	33	7	1				
	4	Fraste Mito Trac			4.57			109	>50														Bentonite Chips	
	5		SAND, fine to coarse sand; grey; moist, very dense.		4.07	3	55	198																
	6					4	SS	100	>50			0												- 
	7					5	GS				0													
			End of Borehole at target depth.		7.62		$\left  \right $													$\vdash$				
	8																							-
																								-
																								-
	9																							
																								-
																								-
	10																							_
			* Note: Split-Spoon sampling and recorded blow		were com		1							SOIL		SSIFI			YSTI	<u> </u> Ем: с				REV:
	DEI 1 ·	PTH	SCALE within ali-rotary or sonic cored borehole, a me strictly comply with ASTM standards. Caution therefore be exercised in the interpretation of counts and their correlation with standard "N"	thod will and ju the mea values.	hich does n dgment sho asured blow	ot ould v		ţ		G	OL	. D	ER			5011	5411	د <b>د</b> .				- 		0
1 PN	1 :	50	counts and their correlation with standard "N"	vaiuëS.					· .									C	CHEC	KED	: JDS	3		_
PF		ECT No.: 20143647 / 2000 T: CP Rail		F	REC	:0	RD	0	FBC	REH	IOLE	E: BH2	0-09							SI	HEET	1 OF 2 : Ground Surface		
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PF	OJE	ECT: Cascadia Phase 4 Viterra East TON: Mile 123.14							DRILLI DRILLI	NG DA NG CO	TE: Ju NTRA	ly 9, 2020 CTOR: Ge	otech D	rillinc	ser	/ices	Ltd.			2.				
N: Not GP	~546 e: Nor S in th	60031 E: ~500440 thing and Easting Coordinates have been determined by e field and are approximate only.				INC	LINA	ATIC	DN: -90	0								SA	AMPL	ERH	HAMM	ER, 64kg; DROP,	762mm	
щ	U				5	SAM	PLES	;	W	ATER C		IT PERCEN	IT VI	CL/	GRA	DATI	ON % IZE <= 0	.002	≅X %	NT %	٩Ľ	PIEZOMET STANDPIF	ER, PE	
DEPTH SCA METRES	DRILLING R		STRATA PLOT	ELEV. DEPTH (m)	NUMBER	түре	RECOVERY %	BLOWS/0.3m	SHEAF Cu, kP	ρ 2 R STREM a	<u>р</u> идтн ю	NP - Non 30 44 nat V. + rem V. ⊕ Pocket F 60 80	Q - ● U - ● Pen - ■	GRAVEL	SAND	FINES	SILT	CLAY	PLASTICITY IND	ORGANIC CONTE	ADDITIONA LAB. TESTIN	OR THERMIST INSTALLAT	OR ION	
— o		Ground Surface	××	> 0.00			_																KOXOXI	
- - - - - - - - - - - - - - - - - - -		(SM) gravelly SILTY SAND, fine to coarse sand, fine to coarse sub-rounded gravel; brown; dry, inferred loose to compact.		0.61	-																	Gravel		
		(CL-ML) sandy gravelly SILTY CLAY	×	1.83	1A	SS	79	10																
2		to CLAYEY SILT, fine to coarse sand, fine to coarse sub-rounded to sub-angular gravel; greyish brown, with iron oxide staining; moist, very soft to soft.			2	55	4	0		F	-01			14	27	59	53	6	7					
- - - - - - - - - - - - - - - - - - -		- wood fibres encountered at approximately 3.7 m depth.				55	-																	
	Frack Mounted Drill	(GC) sandy CLAYEY GRAVEL, fine to coarse gravel, fine to coarse sand; brownish grey, with iron oxide staining; wet, compact.		4.27	3	SS	67	24		0	н			39	31	30	21	9	4					
	Fraste Mito	<ul> <li>(CI, CL and ML) SILT and SILTY CLAY, low plasticity, some fine to medium sand; grey, with 1-10 mm thick light grey laminations; w&gt;PL, inferred stiff to very stiff.</li> </ul>		5.18	40																	Bentonite Chips		
		INFERRED BEDROCK - (SM) SILTY SAND, fine to coarse sand, some fine to coarse sub-rounded to sub-angular gravel; grey; moist, dense to very dense		6.40	4B	SS	100	47																
8					5	SS	58	40						44	36	20							- - - - - - - - - - - - - - - - - - -	
					6	SS	17	>100												-				
		CONTINUED NEXT PAGE																			Ļ	l,		
DE 1	PT⊦ : 50	• vote: - spit-spoon sampling and recorded blow within air-rotary or sonic cored borehole, a me strictly comply with ASTM standards. Caution therefore be exercised in the interpretation of counts and their correlation with standard "N"	v counts ethod wh and jud the mea values.	were comp hich does n dgment sho asured blow	veted ot ould				G	OL	. D	ER	SUIL	ULA	55IFI	CAII		LOC	EIVI: ( GGEI CKED	D: NF	5 H S		0	

F	PRO	DJEC ENT:	T No.: 20143647 / 2000 CP Rail		F	REO	co	RD	0	F BC	REH	OLE	: BH2	20-09							Sł	HEET ATUM:	2 OF 2 Ground Surface	)
F	PR( .00	DJEC CATIC -5460	T: Cascadia Phase 4 Viterra East N: Mile 123.14 031 E: ~500440							DRILLI DRILLI	NG DAT NG COI	re: Jul Ntrac	y 9, 2020 TOR: Ge	eotech D	rilling	Serv	/ices	Ltd.	5/					762mm
G	lote: GPS	in the fi	ng and Easting Coordinates have been determined by eld and are approximate only.			1	INC	CLIN	ATIC	DN: -90				IT		CPA		201 0/						
ЧГЕ УГЕ		RIG	SOIL PROFILE				SAM	PLES	5	Wp				NI	CL/	AY PART	TICLE SI	JIN 76 ZE <= 0	.002	DEX %	TENT %	AL	STANDPIF	ER, PE
DEPTH SC METRE		DRILLING ME	DESCRIPTION	STRATA PLO	ELEV. DEPTH (m)	NUMBER	TYPE	RECOVERY 9	BLOWS/0.3m	1 SHEAF Cu, kP	0 2 R STREN a	ρ IGTH ο	NP - Nor 30 4 nat V. + rem V. ⊕ Pocket F 60 8	Q - ● U - ● Pen - ■	GRAVEL	SAND	FINES	SILT	СLAY	PLASTICITY IN	DRGANIC CON	ADDITION LAB. TEST	THERMIST INSTALLAT	OR ION
		19		0,				u.		2	-			0										
	1	te Mito Track Mounted Drill Air Rotary	INFERRED BEDROCK - (SM) SILTY SAND, fine to coarse sand, some fine to coarse sub-rounded to sub-angular gravel; grey; moist, dense to very dense (continued)			7	SS	0	91														Bentonite Chips	
Ē		Frast		K																				- 122
	2 3 3 4 6 6 7 7 8																							
	9 20 DEF	PTH S	* Note: Split-Spoon sampling and recorded blov within air-rotary or sonic cored borehole, a me SCALE strictly comply with ASTM standards. Caution	v counts thod wh	were comp ich does n igment shc	bleted ot ould				G				SOIL	CLAS	SSIFI	CATIO	ON S	YSTE		GACS			REV:
1	1:	50	therefore be exercised in the interpretation of counts and their correlation with standard "N"	the mea values.	sured blow	′	<			9		ע.	CR					С	HEC	KED	: JDS	3		0

PR	OJE	CT No.: 20	143647 / 2000		R	EC	ORI	DC	)F S	SON	IC	HOI	.E:	SH	20-	10					SHE	EET	1 OF 3	
PR LO	OJE CAT	CT: Casca ION: Inter	adia Phase 4 Viterra East idal - Mile 123.51						DF DF	RILLIN	G DA G CC	TE: J	uly 27	7, 202 R: Mi	20 ud Ba	av Dri	llina Ca	o. Ltd.			DAT	UN:	Ground Surface	
N: · Note GPS	~545 e: Nort S in the	9739 E: ~ hing and East field and are	-499932 ing Coordinates have been determined by approximate only.				INCL	INAT		-90°						., 2	in ig et	. 2.0.		*SAMP	LER HA	MME	ER, 64kg; DROP,	762mm
щ	U U		SOIL PROFILE			SA	AMPLE	ES	S	OIL CC	RE	CL	GRA AY PAR	DATIO	ON % IZE <= 0	0.002	SHEA Cu, k	AR STR Pa	ENGTH	nat V. + rem V.€	- Q - ● 9 U - ●	.0	PIEZOMETE	ER,
DEPTH SCA METRES	DRILLING RI		DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	түре	BLOWS/0.3m	RUN No.	RECO	VER) ଜ କୁରୁ	GRAVEL	SAND	FINES	SILT	CLAY	WA Wp			Pocke 60 T PERC / IP - Non	t Pen 30 ENT WI -Plastic 10	ADDITIONAL LAB. TESTIN	OR THERMISTO INSTALLATIO	OR ON
- 0		Ground	Surface																20		+0			
		(SP) S some sub-ar shells - with boulde - woo	AND, tine to coarse sand, fine to medium sub-rounded to gular gravel; light brown, with fragments; dry to moist. inferred cobbles and/or ers below 0.6 m depth. d and fibrous organics netered from anoroximately 1.1 to		0.00	2	GS GS		1														Filter Sand	
		1.2 m	depth. M) gravelly SAND, fine to	/	. 1.22	3	GS					25	67	8										
2		coarse sub-ro trace i shell fi (CI, C mediu thick li stiff to	sand, fine to coarse unded to sub-angular gravel, non-plastic fines; brown, with agments; moist. L and ML) SILTY CLAY, low to m plasticity; grey, with 1-10 mm ght grey laminations; w>PL, very hard.		1.68	4	SS	15	2															-
						5	SS	20												s	u=267.1	€		
						6	GS		3			0	0	100	79	21			<b></b>	s -	r=133.6			
- 4																								-
•	nted MiniSonic	0				7	SS	27																
- 5	LS250 Track Mour					8	GS		4														Bentonite Chips	
- 7						9	GS		5									F		•	427.4	-		
- 8						10	SS GS	29	6															
- 10			CONTINUED NEXT PAGE		 	_12_	GS		7				 								€ 200.3	• —		
DE 1 :	РТН 50	SCALE	* Note: Split-Spoon sampling and recorded within a sonic cored borehole, a method wh with ASTM standards. Caution and judgme exercised in the interpretation of the measu correlation with standard "N" values.	blow cou ich does nt should red blow	nts were co not strictly o d therefore l counts and	ompleted comply be I their			(	GC		. C	) E	R	SC	DIL C	LASSI	-ICATI	ON SY CH	stem: Logge Ieckee	GACS D: BB D: JDS		F	REV: <b>0</b>

PR		ECT No.: 2	0143647 / 2000		R	EC	ORI	DO	F S	501	NIC	; H	IOL	E:	SH	20- <sup>-</sup>	10					SHE	EET	2 OF 3	
PR LO		ECT: CP Rail ECT: Casc FION: Inter	adia Phase 4 Viterra East tidal - Mile 123.51						DR				E: Ji itra	uly 27	7, 202 R∙ Mi	20 ud Ba	v Dri	lling Cr	) I td			DA	I UM:	Ground Surface	
N: Note GPS	~54 e: No S in t	59739 E: rthing and Eas	~499932 ing Coordinates have been determined by approximate only.				INCLI	NAT		-90°	,	501		010	IX. IVI		iy Dh		). Liu.		*SAMF	PLER HA	MM	ER. 64ka: DROP. 762r	nm
щ	U	다. 이 아이	SOIL PROFILE			S/	AMPLE	ŝ	so	DIL C	ORE		CLA	GRA	DATIO	ON % ZE <= 0	.002	SHEA Cu, k	R STF Pa	ENGTH	Inat V rem V.	+ Q-● ∌ U-●		PIEZOMETER, STANDPIPE	
DEPTH SCA METRES	DRILLING RI	DRILLING MET	DESCRIPTION	STRATA PLOT	ELEV. DEPTH (m)	NUMBER	түре	BLOWS/0.3m	RUN No.	REC	8 6 8 %	RY R	GRAVEL	SAND	FINES	SILT	CLAY	W/ Wp			Pocke 60 IT PERC V NP - Nor 30	et Pen ■ 80 CENT -I WI 1-Plastic 40	ADDITIONAL LAB. TESTING	OR THERMISTOR INSTALLATION	
10 		(CI, C mediu thick stiff to	L and ML) SILTY CLAY, low to im plasticity; grey, with 1-10 mm ight grey laminations; w>PL, very hard. (continued)																						
- 11 		- 50 I grave	nm sandstone clast with some i encountered at 11.9 m depth.			13	GS		7				0	3	97	69	28				-4				CARACARARARARARARARARA
- - - - - - - - - - - - - - - - - - -						14	SS	39																	
									8																
- 14 	ck Mounted MiniSonic	Sonic				16	GS																	Bentonite Chips	IN THE REPORT AND A DAY OF A D
- - - - - - - - - - - - - - - - - - -	LS250 Tra					17	GS		9												Ð	333.5	3		<u> </u>
- 17 - 17 - 18 - 18						18	GS													F	<b>)  </b>	5u=480.8			<u> </u>
- - - - - - - - - - - - - - - - - - -						19	GS		10									   				Sr=200.3	•		
DE 1 :	PTI 50	I SCALE	* Note: Split-Spoon sampling and recorded within a sonic cored borehole, a method wh with ASTM standards. Caution and judgme exercised in the interpretation of the measu correlation with standard "N" values.	blow cou ich does nt should red blow	Ints were co not strictly o d therefore I counts and	omplete comply be their			(	5 (	0	L	D	E	R	SC		LASSI	I FICAT	ON SY	STEM: LOGGI	GACS ED: BB D: JDS		REV	)

PF			Г No.: 20143647 / 2000 СР Рајј		R	EC	OR	DC	)F S	SO	NI	CH	IOL	.E:	SH	20-′	10					SHE		3 OF 3	
PF		NT: IECT	CP Rall F: Cascadia Phase 4 Viterra East N: Intertidal - Mile 123.51						DF			DAT	E: J	uly 27	7, 202	0 Id Po			1 + 4			DAI	UN:	Ground Surface	,
N: No	~54 te: No	4597 Iorthin	739 E: ~499932 g and Easting Coordinates have been determined by				INCL	ινίδτ		-90	ING P	CO	N I FOA		rt. Ivit	и Ба	y Dhi		. Llu.		SAMP				762mm
ш	0	ціе ле	SOIL PROFILE			S/	AMPLI	ES	s	OIL	COR	E	CL/	GRA AY PART	DATIO	DN % ZE <= 0.	.002	SHEA Cu. kF	R STRI 'a	ENGTH	nat V. + rem V.€	• Q - ● • U - ●		PIEZOMETI STANDPIE	ER,
DEPTH SCAL METRES	<b>DRILLING RI</b>	ILLING METH	DESCRIPTION	RATA PLOT	ELEV. DEPTH	NUMBER	ТҮРЕ	OWS/0.3m	RUN No.	RE	COV %	ERY	GRAVEL	SAND	FINES	SILT	CLAY	2 WA Wp	0 4 TER CO		Pocket	t Pen 30 ENT WI Diagtic	ADDITIONAL LAB. TESTING	OR THERMISTO INSTALLATI	DR ION
		R		STF	(m)	2		BL	_	80	<u> </u>	50						1	0 2	20 3	P - Non- 30 4	-Plastic 10			
_ 20	nic		(CI, CL and ML) SILTY CLAY, low to							-	$\square$														- 2222
	ted MiniSc		medium plasticity; grey, with 1-10 mm thick light grey laminations; w>PL, very stiff to hard. <i>(continued)</i>																						
	ack Moun	Sonic				20	GS		10															Bentonite Chips	
- 21 -	LS250 Tra																								
-			End of Sonic Hole at target depth.		21.34																				
- 22																									-
																									-
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- 23																									-
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- 28																									- 
				1																					-
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- 29				1																					-
-																									-
-  -  -				1																					-
30				1																					_
		<u> </u>	* Note: Split-Spoon sampling and recorded b within a sonic cored borehole, a method whic	low cou	nts were co not strictly o	mplete comply	ď			~						SC	OIL CI	LASSIF	ICATIO	DN SYS	STEM: (	GACS			REV:
1	: 50	0	with ASTM standards. Caution and judgmen exercised in the interpretation of the measure correlation with standard "N" values.	t should ed blow	therefore t counts and	be their			(	J	U		D.		. R	2				L CH	LUGGE ECKED	D: BB D: JDS			0

PR	OJI EN	ECT T:	T No.: 20143647 / 2000 CP Rail		R	EC	OR	D C	DF S	SO	NIC	C F	IOL	.E:	SH	20-	11					SH DA	IEET	1 OF 2 : Ground Surface	
PR LO	OJI CAT	ECT TIO	T: Cascadia Phase 4 Viterra East N: Intertidal - Mile 123.35 844 E: ~500134						DF DF	RILLI RILLI	NG NG	DAT CON	TE: J	uly 28 CTO	3, 202 R: Mi	20 ud Ba	ay Dril	lling Co	o. Ltd.						
Note GPS	: No	he fie	g and Easting Coordinates have been determined by eld and are approximate only.			-	INCL	INAT	10N:	-90	0		-	CRA		<u>201 %</u>			Deto		*SAM	PLER H	AMM	ER, 64kg; DROP, 76	<u>32mm</u>
ы S Ы Е	RIG	THO	SOIL PROFILE	F		S.	AMPL	ES	s	OIL	CORI	E	CL	GRA AY PAR	TICLE S	JN % IZE <= 0	0.002	Cu, k	Pa	ENGIH	nat v. rem V. Pock	+ Q- .⊕ U- (et Pen	AL	STANDPIPE	-,
TH SC ETRE	UNG	NG ME		A PLO	ELEV.	BER	E	S/0.3n	No N	REC		ERY	4VEL	QN	VES	5	γ	WA	20 ATER C	40 ONTEN	60 T PER	80 CENT	DITION . TEST	THERMISTOR	ł N
ΔED	DRIL	RILLI	DESCRIPTION	TRAT	DEPTH (m)	NUN	∣≿	BLOW	RUN	~	%	~	GR/	SA	Ē	l ∞	U U	Wp	<b>⊢</b>		/ IP - No	- WI on-Plastic	ADI		
			Ground Surface	S							<u>8 4</u>								10 :	20 :	30	40	+		
- 0			(GP) GRAVEL, fine to coarse sub-rounded to sub-angular gravel,	00	0.00	1	GS																		
			some fine to coarse sand; brown, with shell fragments; non-cohesive, moist.	/•\)	0.30																			Filter Sand	
			(GP-GM) sandy GRAVEL, fine to coarse sub-angular to sub-rounded	Pa		2	GS	1	1				61	32											<b>33</b>
- 1			gravel, fine to coarse sand, some fines; light brown to orange;	$\Lambda$	0.76	3	GS												H	ᆗ					
			(ML) SILT, some fine to coarse sand,	'																					
			moist.		1.37																				
			CLAYEY SILT, low to medium plasticity: grey, with 1-10 mm thick																						
2			light grey laminations; w>PL, very stiff to hard.																						
						4	GS	-	2				0	0	100	80	20								
						<u> </u>								Ū			20								
3										-		-										347	2		
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						5	GS		3																
4																									
	DIC																								
	MiniSo											1									⊕	387	3		
5	ounted	onic																							
	rack M	й																						Dontonito China	
	S250 T		<ul> <li>sand layer encountered from 5.3 to 5.5 m depth.</li> </ul>			6	69	-	4															Bentonite Chips	
	2					0	00																		
6																									
																				₽		360	6		
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10	_	_			4	_9_	<u>G</u> S	↓_	┝┤	-  -	Ц.	╡-	<u> </u>	<u> </u>		<u> </u>		<b>├</b>	┤──┙	<u>н</u>	<u> </u>	. +	4-		
			CONTINUED NEXT PAGE																						
DEI	PTł	чs	* Note: Split-Spoon sampling and recorded within a sonic cored borehole, a method wi with ASTM standards. Caution and iudome	blow cou nich does ent shouk	ints were co not strictly of therefore i	omplete comply be	ď		(	2	$\frown$		Г			so	JIL CI	LASSI	-ICATI	ON SYS	STEM	I: GACS GED: JF			:v:
1:	50	)	exercised in the interpretation of the measu correlation with standard "N" values.	ired blow	counts and	their										<b>`</b>				СН	ECKE	ED: JDS			U

PF	201	JEC	T No.: 20143647 / 2000		R	EC	OR	D C	DF S	501	NIC	; H	IOL	E:	SH	20- <sup>-</sup>	11					SHE	EET	2 OF 2	
CL PF LC	LIEF ROJ DCA	NT: JEC <sup>-</sup> ATIO	CP Rail T: Cascadia Phase 4 Viterra East N: Intertidal - Mile 123.35						DR				E: Ju	uly 28	3, 202	20 Id Do			1 + 4			DAT	UM:	Ground Surface	
N: Not	~5 te: \	4598 Jorthin	844 E: ~500134 Ing and Easting Coordinates have been determined by eld and are approximate only				INCL	ινίατ			, ,	JUN	IIRA		R: IVI	ла ва	y Dri	lling Co	. Lta.	*	SVWD	ЕВ НА			762mm
ш	0	, p	SOIL PROFILE			S	AMPLI	ES	SC	DIL C	ORE		CLA		DATIO	ON % ZE <= 0	.002	SHEA Cu. kF	R STRE	ENGTHr	nat V. +	Q - • U - •		PIEZOMETE STANDPIP	ER,
SCAL	NG RIC	METH		PLOT		н		).3m	ö				EL	0	s		~	2	0 4	ι0 €	Pocket	Pen 📕	TONAL	OR	DR
DEPTH	RILLI	ILLING	DESCRIPTION	RATA F	DEPTH	JUMBE	ТҮРЕ	O/S//O	SUN N	REC	OVE %	RY	GRAV	SANI	FINE	SILT	CLA	WA Wp	TER CO		PERCI	ENT WI	ADDIT LAB. TI	INSTALLATI	ON
		, R		STF	(m)	2		BL	-	8	8 4	50						1	0 2	20 3	P-Non- 60 4	Plastic 0			
	Track Mounted MiniSonic	Sonic	(CI, CL and ML) SILTY CLAY and CLAYEY SILT, low to medium plasticity; grey, with 1-10 mm thick light grey laminations; w>PL, very stiff to hard. (continued)			10	GS		6				0	0	100	91	9							Bentonite Chips	
- 13 					15.24	11	GS		7																
			End of Sonic Hole at target depth.	Dilaw col	Ints were co	mplete	đ									sc	DIL CI	LASSIF	ICATIC	DN SYS	STEM: 0	GACS			
DE 1	: 5	ГН S 60	CALE within a sonic cored borehole, a method wh with ASTM standards. Caution and judgme exercised in the interpretation of the measu correlation with standard "N" values.	ich does nt shouli red blow	d therefore l	comply be their			(	30	0	L	D		R	)	0			CHI		D: JF			0

APPENDIX C

Hammer Penetration Test Energy Measurement Results



Rig:C05-033Hammer:GE09-028Calibration Date:10-Jul-2020

					STAN	IDARD PI	ENETRATION	N TEST ENER	GY HAMN	IER CALIBI	RATION RI	ESULTS						
Test Start Depth (ft)	Date	Instrumented Rod Type	Instrumented Rod Area (cm <sup>2</sup> )	Rod String Type	Strain Gauge #1 Serial No.	Strain Gauge #2 Serial No.	Accelerometer #1 Serial No.	Accelerometer #2 Serial No.	Recorded Blow Counts 0"-6"	Recorded Blow Counts 6"-12"	Recorded Blow Counts 12"-18"	N <sup>1</sup> (Blow Counts)	Number of Blows Analyzed	Average Max Force (kN)	Average Max Energy (kN-m)	Average Energy Transfer Ratio (FV) (%)	Standard Deviation of ETR (%)	Refer to Notation Number
104.0	10-Jul-2020	NWJ	9.29	NWJ	515NWJ2	515NWJ1	K5741	K5742	48	83	>59	>142	142	263.9	0.4315	90.9	3.2	3
109.0	10-Jul-2020	NWJ	9.29	NWJ	515NWJ2	515NWJ1	K5741	K5742	23	69	113	182	182	268.7	0.4070	85.8	4.9	
114.0	10-Jul-2020	NWJ	9.29	NWJ	515NWJ2	515NWJ1	K5741	K5742	23	69	>100	>169	169	268.1	0.4134	87.1	5.1	4
Average <sup>2</sup> :														266.9	0.417	87.9		
Standard Deviati	on <sup>2</sup> :													2.1	0.010	2.2		

1. Blow counts from the first 6" were not included.

2. Average and standard deviation are calculated using averaged data from SPT hammer blows from the 6" to 18" interval (ie. N value) from all sampling depths tested.

3. Sampler refused at 15".

4. Sampler refused at 17".

APPENDIX D

### Laboratory Testing

### WATER CONTENT DETERMINATION

ASTM D 2216

GOLDER

Client:	CP Rail
Project:	Phase 4 Viterra
Location:	Burnaby, BC
Project No.:	20143647 Phase: 2000

Sample	Sample	Specimen	Depth	Interval	Water
Location	No.	No.	Depth (m)	Bottom (m)	Content (%)
BH20-01	Composite	2B	1.98	2.13	25.8
BH20-01	4		3.96	4.57	10.9
BH20-01	5		4.57	5.18	6.8
BH20-02	2		1.52	2.13	11.8
BH20-02	4		3.05	3.66	17.3
BH20-02	6		4.27	4.42	15.3
BH20-02	7		6.10	6.71	11.2
BH20-03	1		0.76	1.37	33.8
BH20-03	2		1.52	2.13	25.3
BH20-03	3	3A	3.05	3.35	15.5
BH20-04	Composite	2B	1.83	2.13	25.6
BH20-04	5		4.57	5.18	29.5
BH20-04	6		6.10	6.71	22.6
BH20-04	7		7.62	8.23	31.1
BH20-04	8		9.14	9.75	32.9
BH20-04	9		10.67	11.28	31.5
BH20-04	10		12.19	12.80	28.7
BH20-05	2		3.05	3.66	12.4
BH20-05	3	3A	3.81	4.27	34.4
BH20-05	3	3B	4.27	4.42	26.2
BH20-05	4		4.57	5.18	21.1
BH20-05	5		6.10	6.71	28.3
BH20-05	6		7.62	8.23	26.0
BH20-05	7		9.14	9.75	29.6
BH20-06	1		1.52	2.13	21.3
BH20-06	2		3.05	3.66	18.7
BH20-06	7		7.62	8.23	13.1
BH20-06	8		9.14	9.75	11.4
BH20-07	Composite	3B	4.88	5.18	9.2
BH20-08	2		3.05	3.66	10.6
BH20-08	4		6.10	6.25	21.5
BH20-08	5		6.71	7.32	12.4
BH20-09	1	1B	1.83	2.13	23.4
BH20-09	3		4,57	5,18	16.0

Lab Schedule No.:

SJ Checked

7/22/2020

Date

### WATER CONTENT DETERMINATION

**ASTM D 2216** 

Client: CP Rail Project: Phase 4 Viterra Location: Burnaby, BC Project No.: 20143647 Phase: 2000

GOLDER

Lab Schedule No.:

Sample	Sample	Specimen	Depth	Interval	Water
Location	No.	No.	Depth (m)	Bottom (m)	Content (%)
SH20-10	6		3.81	3.96	27.1
SH20-10	9		6.71	6.86	27.7
SH20-10	13		11.43	11.58	26.0
SH20-10	18		17.53	17.68	27.7
SH20-11	3		0.91	1.07	19.7
SH20-11	4		2.29	2.44	27.3
SH20-11	6		5.49	5.64	31.1
SH20-11	9		9.91	10.06	27.0

r:GINT\_GAL\_NATIONALIM Unique Project ID: Output Form:\_LAB\_WATER CONTENT (REPORT) 2018 JSoquila 8/21/20

8/20/2020 SJ Checked Date



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-01
Project:	Phase 4 Viterra	Sample No.: 2 Specimen: 2B
Location:	Burnaby, BC	Depth Interval (m): 1.98 to 2.13
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

**Preparation Method: Air Dried** 



PLASTICITY CHART

FF	7/10/2020	SJ	7/21/2020
Tech	Date	Checked	Date



		ASTM D 4318
Client:	CP Rail	Borehole ID: BH20-02
Project:	Phase 4 Viterra	Sample No.: 4
Location:	Burnaby, BC	Depth Interval (m): 3.05 to 3.66
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	rks: N/A	

Test Method: A-Multi Point

Preparation Method: Air Dried



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

NE	7/10/2020	SJ	7/21/2020
Tech	Date	Checked	Date



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-03
Project:	Phase 4 Viterra	Sample No.: 1
Location:	Burnaby, BC	Depth Interval (m): 0.76 to 1.37
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

Preparation Method: Air Dried



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

FF	7/10/2020	SJ	7/21/2020
Tech	Date	Checked	Date



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-04
Project:	Phase 4 Viterra	Sample No.: 2 Specimen: 2B
Location:	Burnaby, BC	Depth Interval (m): 1.83 to 2.13
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

Preparation Method: Air Dried



PLASTICITY CHART

NP - NON-PLASTIC RESULT ND - NOT DETERMINED

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

FF	7/10/2020	SJ	7/21/2020
Tech	Date	Checked	Date

Golder Associates Ltd.



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-04
Project:	Phase 4 Viterra	Sample No.: 5
Location:	Burnaby, BC	Depth Interval (m): 4.57 to 5.18
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

**Preparation Method: Wet** 



PLASTICITY CHART

FF	7/21/2020	SJ	7/22/2020
Tech	Date	Checked	Date



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-04
Project:	Phase 4 Viterra	Sample No.: 7
Location:	Burnaby, BC	Depth Interval (m): 7.62 to 8.23
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

Preparation Method: Wet



PLASTICITY CHART

NP - NON-PLASTIC RESULT ND - NOT DETERMINED

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

NE	7/21/2020	SJ	7/22/2020
Tech	Date	Checked	Date

Golder Associates Ltd.



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-04
Project:	Phase 4 Viterra	Sample No.: 9
Location:	Burnaby, BC	Depth Interval (m): 10.67 to 11.28
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	rks: N/A	

Test Method: A-Multi Point

**Preparation Method: Air Dried** 



#### PLASTICITY CHART

NP - NON-PLASTIC RESULT ND - NOT DETERMINED

Dutput

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

FF	7/20/2020	SJ	7/22/2020
Tech	Date	Checked	Date

Golder Associates Ltd.



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-05
Project:	Phase 4 Viterra	Sample No.: 5
Location:	Burnaby, BC	Depth Interval (m): 6.10 to 6.71
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

Preparation Method: Wet



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

FF/DC	7/9/2020	SJ	7/21/2020
Tech	Date	Checked	Date

Golder Associates Ltd.



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-06
Project:	Phase 4 Viterra	Sample No.: 1
Location:	Burnaby, BC	Depth Interval (m): 1.52 to 2.13
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

Preparation Method: Air Dried



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

FF	7/10/2020	SJ	7/21/2020
Tech	Date	Checked	Date

Golder Associates Ltd.



		ASTM D 4318
Client:	CP Rail	Borehole ID: BH20-08
Project:	Phase 4 Viterra	Sample No.: 2
Location:	Burnaby, BC	Depth Interval (m): 3.05 to 3.66
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	rks: N/A	

Test Method: A-Multi Point

Preparation Method: Air Dried



PLASTICITY CHART

EE 7/17/2020 SI 7/22/2020

FF	7/17/2020	SJ	7/22/2020
Tech	Date	Checked	Date



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-09
Project:	Phase 4 Viterra	Sample No.: 1 Specimen: 1B
Location:	Burnaby, BC	Depth Interval (m): 1.83 to 2.13
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

Preparation Method: Air Dried



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

NE	7/20/2020	SJ	7/22/2020
Tech	Date	Checked	Date



**ASTM D 4318** 

Client:	CP Rail	Borehole ID: BH20-09
Project:	Phase 4 Viterra	Sample No.: 3
Location:	Burnaby, BC	Depth Interval (m): 4.57 to 5.18
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

**Preparation Method: Air Dried** 



PLASTICITY CHART

NP - NON-PLASTIC RESULT ND - NOT DETERMINED

Dutput

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

FF	7/17/2020	SJ	7/22/2020
Tech	Date	Checked	Date

Golder Associates Ltd.



	ASTM D 4318
Client: CP Rail	Sonic Hole ID: SH20-10
Project: Phase 4 Viterra	Sample No.: 6
Location: Burnaby, BC	Depth Interval (m): 3.81 to 3.96
Project No.: 20143647 Phase: 2000	Lab Schedule No.:
Other Remarks: N/A	

Test Method: A-Multi Point

**Preparation Method: Wet** 



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

NE	8/14/2020	SJ	8/20/2020
Tech	Date	Checked	Date



	ASTM D 4318
Client: CP Rail	Sonic Hole ID: SH20-10
Project: Phase 4 Viterra	Sample No.: 9
Location: Burnaby, BC	Depth Interval (m): 6.71 to 6.86
Project No.: 20143647 Phase: 2000	Lab Schedule No.:
Other Remarks: N/A	

Test Method: A-Multi Point

**Preparation Method: Wet** 



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

NE	8/13/2020	SJ	8/20/2020
Tech	Date	Checked	Date



	ASTM D 4318
CP Rail	Sonic Hole ID: SH20-10
Phase 4 Viterra	Sample No.: 13
Burnaby, BC	Depth Interval (m): 11.43 to 11.58
20143647 Phase: 2000	Lab Schedule No.:
rks: N/A	
	CP Rail Phase 4 Viterra Burnaby, BC 20143647 <b>Phase:</b> 2000 <b>rks: N/A</b>

Test Method: A-Multi Point

Preparation Method: Wet



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

NE	8/13/2020	SJ	8/20/2020
Tech	Date	Checked	Date



	ASTM D 4318
Client: CP Rail	Sonic Hole ID: SH20-10
Project: Phase 4 Viterra	Sample No.: 18
Location: Burnaby, BC	Depth Interval (m): 17.53 to 17.68
Project No.: 20143647 Phase: 2000	Lab Schedule No.:
Other Remarks: N/A	

Test Method: A-Multi Point

Preparation Method: Wet



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

NE	8/13/2020	SJ	8/20/2020
Tech	Date	Checked	Date



**ASTM D 4318** 

Client:	CP Rail	Sonic Hole ID: SH20-11
Project:	Phase 4 Viterra	Sample No.: 3
Location:	Burnaby, BC	Depth Interval (m): 0.91 to 1.07
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	ks: N/A	

Test Method: A-Multi Point

Preparation Method: Wet



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

DC	8/14/2020	SJ	8/20/2020
Tech	Date	Checked	Date



**ASTM D 4318** 

Client:	CP Rail	Sonic Hole ID: SH20-11
Project:	Phase 4 Viterra	Sample No.: 4
Location:	Burnaby, BC	Depth Interval (m): 2.29 to 2.44
Project No.:	20143647 Phase: 2000	Lab Schedule No.:
Other Rema	rks: N/A	

Test Method: A-Multi Point

Preparation Method: Air Dried



PLASTICITY CHART

NP - NON-PLASTIC RESULT ND - NOT DETERMINED

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

FF	8/11/2020	SJ	8/20/2020
Tech	Date	Checked	Date

Golder Associates Ltd.



	ASTM D 4318
Client: CP Rail	Sonic Hole ID: SH20-11
Project: Phase 4 Viterra	Sample No.: 6
Location: Burnaby, BC	Depth Interval (m): 5.49 to 5.64
Project No.: 20143647 Phase: 2000	Lab Schedule No.:
Other Remarks: N/A	

Test Method: A-Multi Point

Preparation Method: Wet



PLASTICITY CHART

NP - NON-PLASTIC RESULT ND - NOT DETERMINED

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

NE	8/12/2020	SJ	8/20/2020
Tech	Date	Checked	Date

Golder Associates Ltd.



		ASTM D 4318	
Client:	CP Rail	Sonic Hole ID: SH20-11	
Project:	Phase 4 Viterra	Sample No.: 9	
Location:	Burnaby, BC	Depth Interval (m): 9.91 to 10.06	
Project No.:	20143647 Phase: 2000	Lab Schedule No.:	
Other Rema	rks: N/A		

Test Method: A-Multi Point

Preparation Method: Air Dried



PLASTICITY CHART

Note: The test data given herein pertain to the sample provided only. This report constitutes a testing service only.

FF	8/11/2020	SJ	8/20/2020
Tech	Date	Checked	Date



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#### ASTM D 422

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Sample Location: BH20-01 Sample No.: 5



CP Rail

Client:



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**ASTM D6913** 

Sample Location: BH20-02 Sample No.: 1

Depth Interval (m): 0.76 to 1.37



Phase 4 Viterra

CP Rail

Client: Project:

Sample No.:

Sample Location: BH20-02

4



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Phase 4 Viterra

CP Rail

Client: Project: **ASTM D6913** 

Sample No.:

Sample Location: BH20-02

6



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CP Rail Client: Project: Phase 4 Viterra

Location: Burnaby, BC

GOLDER
Sample Location: BH20-02



National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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### **ASTM D 422**



CP Rail

Client:

Location: Burnaby, BC Project No.: 20143647 Phase: 2000 Lab Schedule No.: Legend U.S. Sieve Size (meshes / inch) Hydrometer Size of Opening (inches) 24 12 6 3 1 1/2 3/4 3/8 10 20 40 60 100 200 Particle **USCS Particle Size Scale** Percent Sieve Size Size 100 Passing (USS) (mm) (mm) 6" 152.4 100.0 90 3.5" 88.9 100.0 3" 76.2 100.0 2" 50.8 100.0 80 1 1/2" 38.1 100.0 1" 25.4 100.0 70 3/4" 19.1 100.0 Percent Finer by Mass 1/2" 12.7 100.0 3/8" 9.5 100.0 #4 US MESH 4.75 100.0 2 #10 US MESH 99.4 0.85 #20 US MESH 98.8 0.425 #40 US MESH 94.5 #60 US MESH 0.25 86.1 #100 US MESH 0.15 74.0 #140 US MESH 0.106 64.9 30 #200 US MESH 0.075 57.9 0.0380 44.5 0.0276 41.3 20 0.0180 37.7 0.0108 33.5 10 0.0079 29.3 0.0058 23.7 0 0.0041 20.2 <sup>1</sup>Particle Size (mm) 100 10 0.01 0.001 0.0001 0.0029 17.4 0.0013 10.6 GRAVEL SAND BOULDER COBBLE FINES (Silt, Clay) Fine Coarse Coarse Medium Fine KS/NE 7/13/2020 SJ 7/21/2020 Tech Date Checked Date

National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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# **ASTM D 422**

Sample Location: BH20-03 Sample No.:

3 Specimen: 3A

Depth Interval (m): 3.05 to 3.35



Phase 4 Viterra

CP Rail



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#### **ASTM D 422**

Sample Location: BH20-04 Sample No.: 6

Depth Interval (m): 6.10 to 6.71



Phase 4 Viterra

CP Rail

Sample Location: BH20-04



National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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**ASTM D 422** 

GOLDER

Client:

CP Rail

Sample No.:

Sample Location: BH20-04

9

Depth Interval (m): 10.67 to 11.28 Location: Burnaby, BC Project No.: 20143647 Phase: 2000 Lab Schedule No.: Legend U.S. Sieve Size (meshes / inch) Hydrometer Size of Opening (inches) 24 12 6 3 1 1/2 3/4 3/8 10 20 40 60 100 200 Particle 4 **USCS Particle Size Scale** Percent Sieve Size Size 100 Passing (USS) (mm) (mm) 6" 152.4 100.0 90 88.9 3.5" 100.0 3" 76.2 100.0 2" 50.8 100.0 80 1 1/2" 38.1 100.0 1" 25.4 100.0 70 3/4" 19.1 100.0 Percent Finer by Mass 1/2" 12.7 100.0 3/8" 9.5 100.0 #4 US MESH 4.75 100.0 2 #10 US MESH 100.0 0.85 #20 US MESH 99.9 0.425 #40 US MESH 99.8 #60 US MESH 0.25 99.6 #100 US MESH 0.15 99.4 #140 US MESH 0.106 99.3 30 #200 US MESH 0.075 99.1 0.0345 98.5 0.0247 96.5 20 0.0161 91.7 0.0099 80.2 10 0.0072 72.6 0.0055 56.4 0 0.0040 44.1 <sup>1</sup>Particle Size (mm) 100 10 0.01 0.001 0.0001 0.0029 32.0 0.0013 12.4 GRAVEL SAND BOULDER COBBLE FINES (Silt, Clay) Coarse Fine Coarse Medium Fine KS 7/20/2020 SJ 7/22/2020 Date Checked Date Tech

National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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Phase 4 Viterra



Burnaby, BC

Phase 4 Viterra

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Sample Location: BH20-06 Sample No.: 5 Depth Interval (m): 5.33 to 6.10

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Client: Project:

Location:



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Client:

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### **ASTM D 422**

Sample Location: BH20-06

Sample No.: 7

Sample Location: BH20-07



National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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**ASTM D 422** 

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CP Rail

Client:

Project No.: 20143647 Phase: 2000 Lab Schedule No.: Legend U.S. Sieve Size (meshes / inch) Hydrometer Size of Opening (inches) 24 12 6 3 1 1/2 3/4 3/8 10 20 40 60 100 200 Particle **USCS Particle Size Scale** Percent Sieve Size Size 100 Passing (USS) (mm) (mm) 6" 152.4 100.0 90 88.9 3.5" 100.0 3" 76.2 100.0 2" 50.8 100.0 80 1 1/2" 38.1 100.0 1" 25.4 100.0 70 3/4" 19.1 100.0 Percent Finer by Mass 1/2" 12.7 100.0 3/8" 9.5 100.0 #4 US MESH 4.75 100.0 2 #10 US MESH 100.0 0.85 #20 US MESH 99.2 0.425 #40 US MESH 94.4 #60 US MESH 0.25 86.8 #100 US MESH 0.15 72.9 #140 US MESH 0.106 63.1 30 #200 US MESH 0.075 55.1 0.0356 44.4 0.0264 40.4 20 0.0179 34.1 0.0107 29.6 10 0.0079 25.2 21.4 0.0057 0 0.0041 19.4 <sup>1</sup>Particle Size (mm) 100 10 0.01 0.001 0.0001 0.0029 15.6 0.0013 10.4 GRAVEL SAND BOULDER COBBLE FINES (Silt, Clay) Fine Coarse Coarse Medium Fine KS/NE 7/13/2020 SJ 7/21/2020 Tech Date Checked Date

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CP Rail Client: Project: Phase 4 Viterra Location: Burnaby, BC

Sample Location: BH20-07 Sample No.: 3 Specimen: 3B

Depth Interval (m): 4.88 to 5.03



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Sample Location: BH20-08 Sample No.: 1

Depth Interval (m): 1.52 to 2.13

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CP Rail

Phase 4 Viterra

Burnaby, BC

Client: Project:

Location:



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Phase 4 Viterra

CP Rail

Client: Project: **ASTM D 422** 

Sample Location: BH20-08

Sample No.: 2

Depth Interval (m): 3.05 to 3.66

Date

Project No.: 20143647 Phase: 2000 Legend U.S. Sieve Size (meshes / inch) Hydrometer Size of Opening (inches) 24 12 6 3 1 1/2 3/4 3/8 10 20 40 60 100 200 Particle 4 **USCS Particle Size Scale** Percent Sieve Size Size 100 Passing (USS) (mm) (mm) 6" 152.4 100.0 90 88.9 3.5" 100.0 3" 76.2 100.0 2" 50.8 100.0 80 1 1/2" 38.1 100.0 1" 25.4 95.2 70 3/4" 19.1 94.0 Percent Finer by Mass 1/2" 12.7 90.5 3/8" 9.5 88.4 #4 US MESH 4.75 85.4 2 #10 US MESH 75.6 0.85 #20 US MESH 72.7 0.425 #40 US MESH 68.9 #60 US MESH 0.25 65.1 #100 US MESH 0.15 61.6 #140 US MESH 0.106 59.7 30 #200 US MESH 0.075 58.7 0.0364 53.7 0.0267 49.6 20 0.0182 40.4 0.0113 28.9 10 0.0084 20.9 15.2 0.0061 0.0044 10.1 0 <sup>1</sup>Particle Size (mm) 100 10 0.01 0.001 0.0001 0.0031 7.0 0.0013 3.4 GRAVEL SAND BOULDER COBBLE FINES (Silt, Clay) Coarse Fine Coarse Medium Fine KS 7/20/2020 SJ 7/22/2020

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Phase 4 Viterra

Burnaby, BC

CP Rail

Client: Project:

Location:

#### Sample No.: Specimen: 1B 1

Sample Location: BH20-09

Depth Interval (m): 1.83 to 2.13

Lab Schedule No.:

**ASTM D 422** 

Project No.: 20143647 Phase: 2000 Lab Schedule No.: Legend U.S. Sieve Size (meshes / inch) Hydrometer Size of Opening (inches) 24 12 6 3 1 1/2 3/4 3/8 10 20 40 60 100 200 Particle 4 **USCS Particle Size Scale** Percent Sieve Size Size 100 Passing (USS) (mm) (mm) 6" 152.4 100.0 90 88.9 3.5" 100.0 3" 76.2 100.0 2" 50.8 100.0 80 1 1/2" 38.1 100.0 1" 25.4 91.7 70 3/4" 19.1 85.9 Percent Finer by Mass 1/2" 12.7 79.8 3/8" 9.5 68.7 #4 US MESH 4.75 61.2 2 #10 US MESH 54.2 0.85 #20 US MESH 47.7 0.425 #40 US MESH 41.8 #60 US MESH 0.25 37.2 #100 US MESH 0.15 33.9 #140 US MESH 0.106 31.9 30 #200 US MESH 0.075 30.4 0.0407 29.5 0.0293 27.9 20 0.0189 25.8 0.0112 22.9 10 0.0081 20.5 0.0058 17.7 0.0042 14.1 0 <sup>1</sup>**Particle Size (mm)** 100 10 0.01 0.001 0.0001 0.0030 12.0 0.0013 5.9 GRAVEL SAND BOULDER COBBLE FINES (Silt, Clay) Coarse Fine Coarse Medium Fine KS 7/20/2020 SJ 7/22/2020 Date Checked Date Tech

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Phase 4 Viterra

Burnaby, BC

CP Rail

Client: Project:

Location:

Sample Location: BH20-09 Sample No.: 3

Depth Interval (m): 4.57 to 5.18

**ASTM D 422** 

Sample No.:

Sample Location: BH20-09

Depth Interval (m): 7.62 to 8.23

5



National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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CP Rail Client: Project: Phase 4 Viterra

Location: Burnaby, BC

Project No.: 20143647 Phase: 2000

GOLDER

Sample No.:

Sample Location: SH20-10

3



National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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Phase 4 Viterra

CP Rail



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## **ASTM D 422**

Sample Location: SH20-10 Sample No.: 6

Depth Interval (m): 3.81 to 3.96



Phase 4 Viterra

CP Rail

Project No.: 20143647 Phase: 2000 Lab Schedule No.: Legend U.S. Sieve Size (meshes / inch) Hydrometer Size of Opening (inches) 24 12 6 3 1 1/2 3/4 3/8 10 20 40 60 100 200 Particle 4 **USCS Particle Size Scale** Percent Sieve Size Size 100 Passing (USS) (mm) (mm) 6" 152.4 100.0 90 3.5" 88.9 100.0 3" 76.2 100.0 2" 50.8 100.0 80 1 1/2" 38.1 100.0 1" 25.4 100.0 70 3/4" 19.1 100.0 Percent Finer by Mass 1/2" 12.7 100.0 3/8" 9.5 100.0 #4 US MESH 4.75 100.0 2 #10 US MESH 100.0 0.85 #20 US MESH 99.4 0.425 #40 US MESH 98.7 #60 US MESH 0.25 98.3 #100 US MESH 0.15 97.6 #140 US MESH 0.106 97.2 30 #200 US MESH 0.075 96.8 0.0354 95.4 0.0251 94.4 20 0.0162 90.5 0.0100 77.9 10 0.0073 70.2 0.0054 59.9 0 0.0040 47.6 <sup>1</sup>Particle Size (mm) 100 10 0.01 0.001 0.0001 0.0028 37.3 0.0013 16.5 GRAVEL SAND BOULDER COBBLE FINES (Silt, Clay) Fine Coarse Coarse Medium Fine KS/NE 8/10/2020 SJ 8/20/2020 Tech Date Checked Date

National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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Phase 4 Viterra

Burnaby, BC

CP Rail

Client: Project:

Location:

**ASTM D 422** 

Sample Location: SH20-10 Sample No.: 13

Depth Interval (m): 11.43 to 11.58



Burnaby, BC

Phase 4 Viterra

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Sample Location: SH20-11 Sample No.: 2 Depth Interval (m): 0.46 to 0.61



CP Rail

Client: Project:

Location:

Sample No.:

Sample Location: SH20-11

4



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**ASTM D 422** 

GOLDER

CP Rail

Phase 4 Viterra

Location: Burnaby, BC Project No.: 20143647 Phase: 2000 Lab Schedule No.: Legend U.S. Sieve Size (meshes / inch) Hydrometer Size of Opening (inches) 24 12 6 3 1 1/2 3/4 3/8 10 20 40 60 100 200 Particle 4 **USCS Particle Size Scale** Percent Sieve Size Size 100 Passing (USS) (mm) (mm) 6" 152.4 100.0 90 88.9 3.5" 100.0 3" 76.2 100.0 2" 50.8 100.0 80 1 1/2" 38.1 100.0 1" 25.4 100.0 70 3/4" 19.1 100.0 Percent Finer by Mass 1/2" 12.7 100.0 3/8" 9.5 100.0 #4 US MESH 4.75 100.0 2 #10 US MESH 100.0 0.85 #20 US MESH 99.9 0.425 #40 US MESH 99.9 #60 US MESH 0.25 99.9 #100 US MESH 0.15 99.9 #140 US MESH 0.106 99.9 30 #200 US MESH 0.075 99.8 0.0350 98.5 0.0253 94.6 20 0.0170 83.0 0.0107 63.7 10 0.0081 45.3 0.0060 30.9 0 0.0044 17.7 <sup>1</sup>Particle Size (mm) 100 10 0.01 0.001 0.0001 0.0031 13.4 0.0013 5.4 GRAVEL SAND BOULDER COBBLE FINES (Silt, Clay) Fine Coarse Coarse Medium Fine KS/NE 8/10/2020 SJ 8/20/2020 Tech Date Checked Date

National IM Server: GINT\_GAL\_NATIONALIM Unique Project ID:2563 Output Form: LAB\_PARTICLE SIZE (W/ GRADATIONS) 2018 JSoquila 8/21/20

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Phase 4 Viterra

CP Rail

Client: Project:

Sample No.: Depth Interval (m): 9.91 to 10.06

Sample Location: SH20-11

9

**ASTM D 422** 



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